ADDENDUM #2

REQUEST FOR PROPOSALS:
GENERAL CONTRACTOR SERVICES FOR THE PUBLIC WORKS COMPLEX

The following changes are hereby made to the Contract Documents for the above referenced Request for Proposals:

1. **PROPOSAL DUE DATE CHANGE:** Proposals are due by **Tuesday, April 23, 2024 @ 3:00pm**. All other dates are to remain unchanged as shown in Addendum #1.
2. Any clarifications provided in the Pre-Proposal Meeting Minutes (dated March 28, 2024) are herein incorporated and made part of the proposal requirements.
3. The 14-month maximum listed in the RFP is eliminated. Contractors are expected to create and propose their own schedule beginning on the estimated Notice to Proceed date of May 22, 2024. The proposed schedule is part of the proposal evaluation criteria.
4. **Attachment D – Section 1 Operations Building (Project Drawings)** have been updated and a copy is attached. The following sheets have been revised and shall replace the original sheets: Sheets A311, A312, A313, A601, S002, S101A, S101B, S101C, S111A, S111B, S121A, S121B, S121C, S511, S521, S522, S601, S602, and S603. Changes have been clouded.
5. **Attachment D – Section 2 Vehicle Storage Building (Project Drawings)** have been updated and a copy is attached. The following sheets have been revised and shall replace the original sheets: Sheets S101, S601, and S602. Changes have been clouded.
6. **Attachment D – Section 3 Site Civil (Project Drawings)** have been updated and a copy is attached. The following sheets have been revised: Sheets C4.0, C5.1 and C6.0. Changes have been clouded and include extending the sidewalk by the main entrance into the site to provide a walkway path to the existing building, adding an ADA ramp, crosswalk striping, and striping to add a van accessible ADA parking stall.
7. The cost for Section 3: Site Civil shall include all conduit and wiring for power and communications between the Operations Building and Vehicle Storage Building to all other site components (e.g. yard lights, gate opener, gate motor, etc.). Use 1-1/2" conduit where not specified otherwise.
8. A 5% Bid Bond of the Total Project Cost submitted in the revised **Attachment B** is required as part of the submitted proposal (electronic version is acceptable).
9. Material Substitution Request: MasterLife 300 is approved as equal to Xypex Admix C-1000 as a waterproofing admixture wherever listed in **Attachment D** (all sections).
10. **Change in Scope**. If a change in the scope of the project is required due to inadequate funding for the entire project, the city will determine the process for proceeding

forward based on the magnitude of the change in scope required. The following are options that the city may implement:

a. If the needed change in scope is relatively small and Value Engineering (VE) suggestions were included with the proposal, the city may request an exact cost for each VE item identified. If accepted by the city, the cost savings would be applied, thus adjusting the contractor's bid amount. The Project Bid portion of the evaluation criteria would then be re-evaluated, and scoring adjusted accordingly. If selected, the contractor would be required to honor the VE cost savings proposed. A concurrent change order for the new bid amount would accompany the recommendation to award.
b. If the needed change in scope is significant, the city will use the evaluation criteria and scoring to select the top 2 or 3 contractors based on their original proposals. A total "not to exceed" amount will be given and the contractors will be expected to propose a new adjusted scope of work that could be completed for the amount given. The city will then determine which revised scope of work is the most advantageous and make their recommended selection accordingly. A concurrent change order for the new bid amount would accompany the recommendation to award.

11. The cost for the entrance fencing, gates, gate opening system, and gate opener should NOT be included in the bid. The selected contractor will provide material options and costs for these items for the city to review and approve after the contract has been awarded. Proceeding with a selected option will be handled as a change order.
12. Section 4.03I (page 40 of Attachment D, Section 3) indicates that all costs associated with the testing required by the specifications for the site civil work items is the responsibility of the contractor. This is being revised, as follows: All costs associated with the testing required by the specifications for the site civil work items is the responsibility of the city. The contractor is still responsible for quality control and all costs associated with that effort, as deemed necessary. The costs for video inspection of all new or relocated gravity lines (sewer, storm drain, and land drain) is still the responsibility of the contractor.
13. Material Substitution Request: Draper FlexShade NEXD is approved as an equal for the Manual Roller Shade as specified in Division 12 Furnishings, Section 12 2413, Page 3, Paragraph 2.2 Products.
14. Material Substitution Request: The Westcoat Specialty Coating System, Dubro System with an added layer of EC-44 Flex Epoxy is an acceptable substitution for the specified FL-05 Floor System.
15. Material Substitution Request: DKS Doors and Frames is an acceptable substitution for the specified doors and frames identified in Specification Section 08 1113.
16. **Attachment B – Bid Form** has been *revised* to include a total for the Construction

Management and a total for the Project. This version of the form must be included in the submitted proposal.

This Addendum is hereby attached to and made part of the Request for Proposals documents and each Proposer shall acknowledge receipt of this Addendum on the response.

DocuSigned by:
Adam Favero
2F94C66ECFFC46C...

April 16, 2024

Adam Favero – Public Works Director

ATTACHMENT B
BID FORM

Company: _____

CONSTRUCTION MANAGEMENT (CM) BID (Lump Sum)

Item	Cost $	Cost Written*
Construction Management	$_____	_____
Cost of Bonds	$_____	_____
Construction Supervision	$_____	_____
Total CM	**$_____**	**_____**

Fee Percentage		
Overhead and Profit Fee Percentage on Self-Performance *(will not be made public)*	_____%	_____percent
Overhead and Profit Fee Percentage on Subcontractors / Procurement *(will not be made public)*	_____%	_____percent

BUILDING CONSTRUCTION & SITE CIVIL BID (Lump Sum)

Contractor ***MUST* also submit a separate Schedule of Values for EACH Section.**

Item	Cost	Cost Written*
Section 1: Operations Building	$ _____	_____
Section 2: Vehicle Storage Building	$ _____	_____
Section 3: Site Civil *(including Decant and Material Storage Structures)*	$ _____	_____
Total Construction	**$ _____**	**_____**

TOTAL PROJECT	**$_____**	**_____**

**In case of discrepancy, written amount shall govern.*

***Performance, payment, and warranty bonds shall be required for the entire Project. Example bonds can be found in Attachment C.*

BIDDER:_____ **Submittal Date: _____**

(Indicate correct name of bidding entity) **License Number: _____**

BY: _____

Signature: _____

Title: _____

ATTEST: _____

Signature: _____

Title: _____

(If Bidder is a corporation, a limited liability company, a partnership, or a joint venture, attach evidence of authority to sign.)

Address for Giving Notices: _____

_____

Phone: _____

POINT OF CONTACT FOR PROJECT

Name: _____

Title: _____

Email: _____

Phone: _____

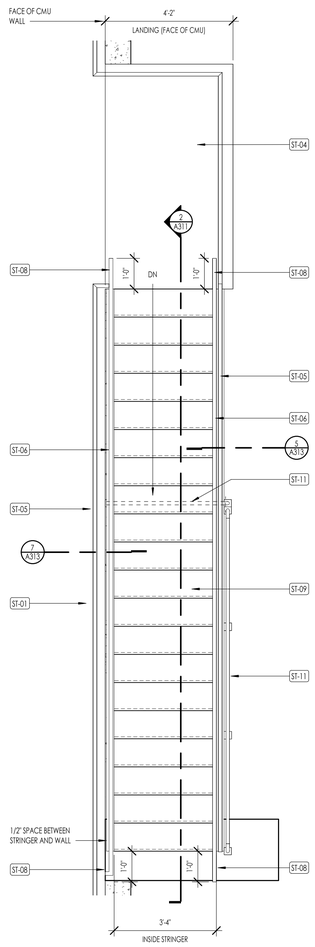
Is the Point of Contact authorized to sign documents on behalf of the Bidding Entity? ☐ **YES** ☐ **NO** *(If no, please complete information below)*

AUTHORIZED SIGNATORY

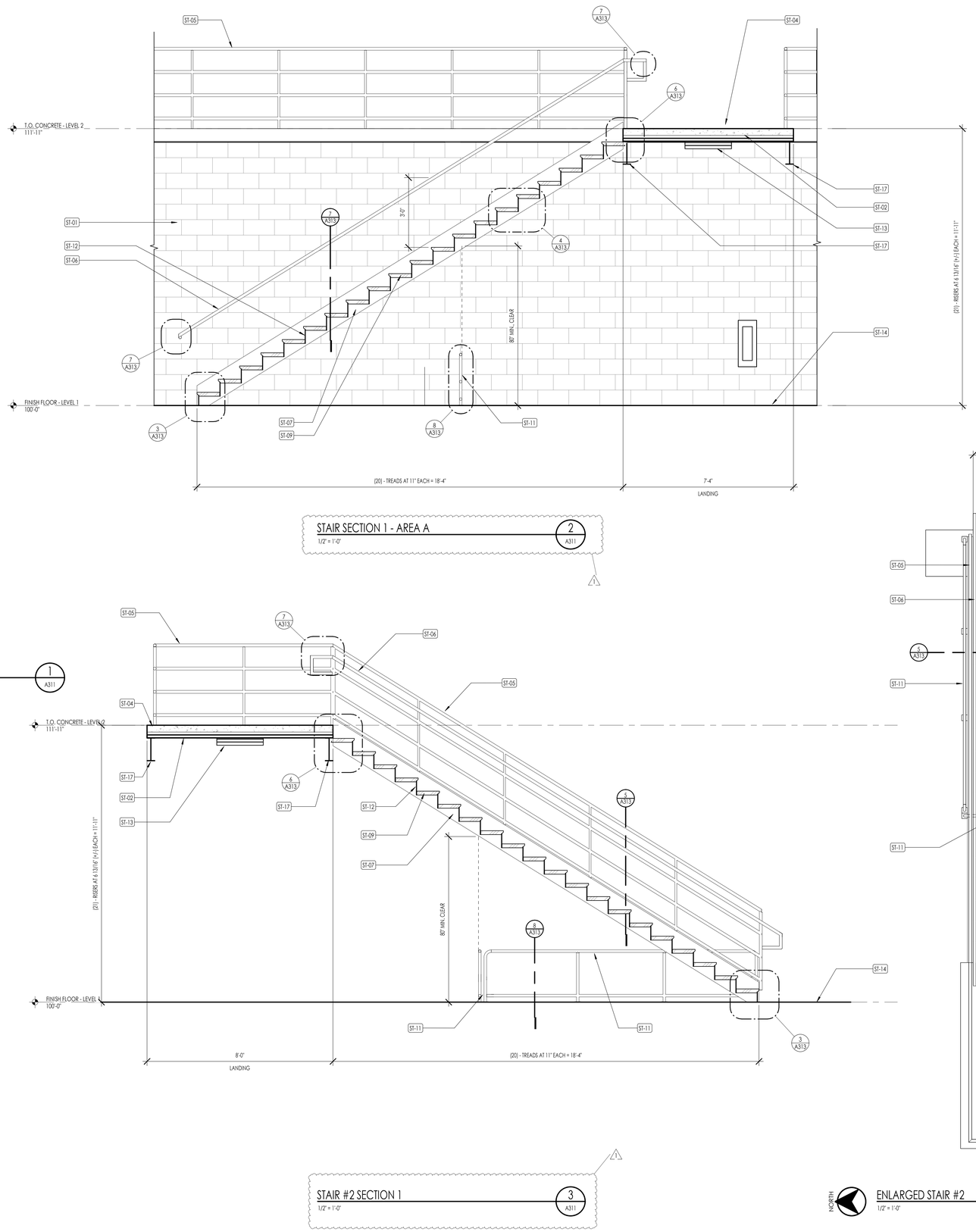
(If different from the point of contact listed above)

Name: _____ **Email:** _____

Title: _____

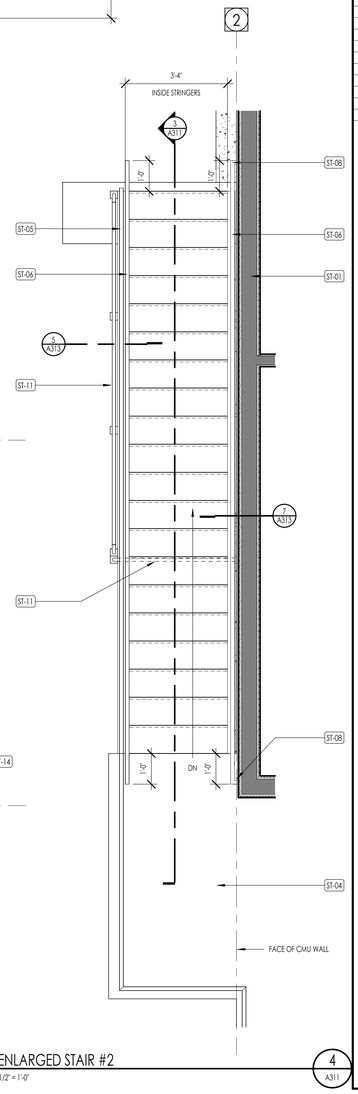


ENLARGED STAIR #1
1/2" = 1'-0"
1 A311



STAIR SECTION 1 - AREA A
1/2" = 1'-0"
2 A311

STAIR #2 SECTION 1
1/2" = 1'-0"
3 A311



NORTH
ENLARGED STAIR #2
1/2" = 1'-0"
4 A311

STAIR PLAN GENERAL NOTES

STAIR PLAN KEYNOTES (ST)

KEYNOTES - STAIRS

ST-01	SCHEDULED WALL TYPE
ST-02	SCHEDULE CEILING
ST-04	FLOOR STRUCTURE
ST-05	42" HIGH GUARDRAIL, ON OPEN SIDE OF STAIRS - PAINTED
ST-06	36" HIGH HANDRAIL, ON BOTH SIDES OF STAIRS - PAINTED
ST-07	HSS 12" X 3" X 1/4" STEEL STAIR STRINGER - PAINTED
ST-08	RETURN HANDRAIL TO FLOOR, GUARD OR WALL.
ST-09	40" (VIF) X 12" 11 GUAGE PRE-GALVANIZED STEEL TREAD W/ TRACTION-TREAD NON-SLIP SURFACE
ST-11	27" HIGH PROTECTION RAIL BENEATH STAIR
ST-12	PRE-GALVANIZED PERFORATED METAL RISER
ST-13	LIGHT FIXTURE
ST-14	SCHEDULE FINISH FLOOR
ST-17	STRUCTURAL BEAM - PAINTED

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

PROJECT NO. 22-111
DATE: MARCH 4, 2024

REVISIONS:

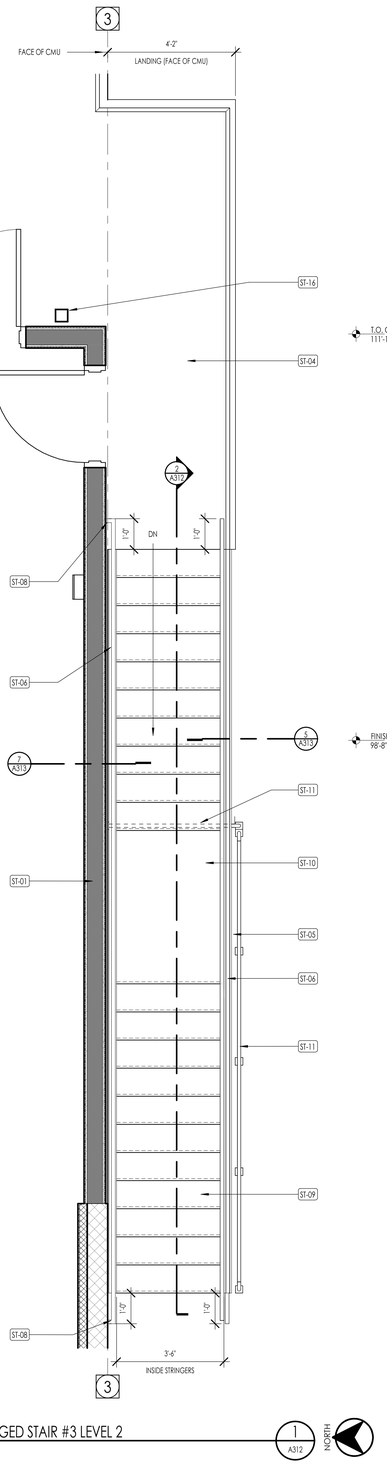
1 4.15.2024 ADDENDUM

BID SET - SECTION 1

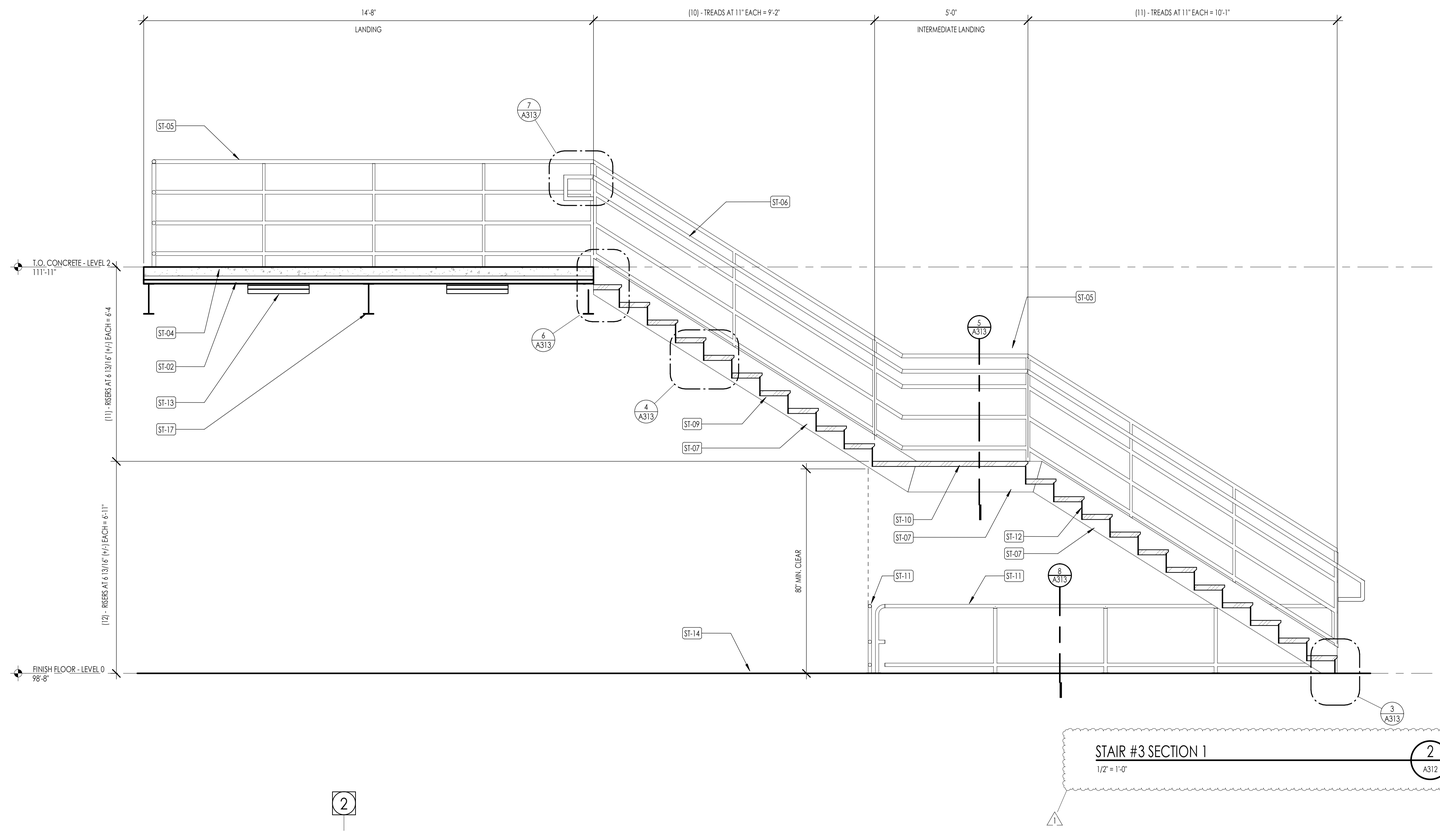
SHEET TITLE:
ENLARGED STAIR PLANS/ SECTIONS

SHEET NUMBER:
A311

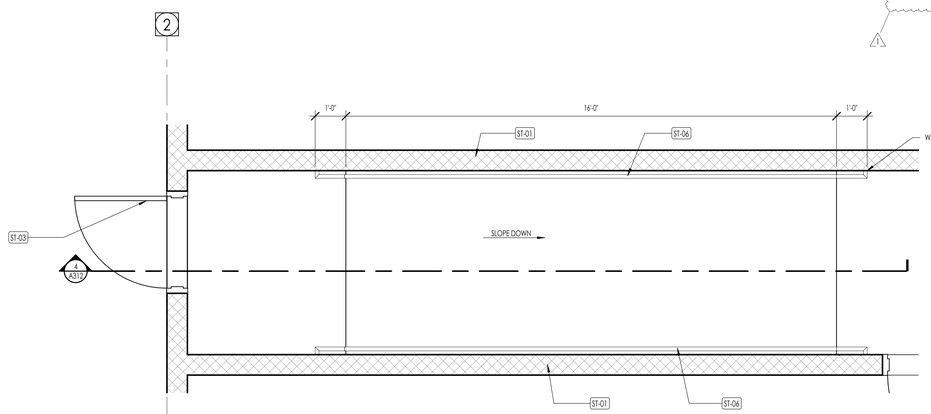
Plot Date: 4/15/2024 12:04:26 PM



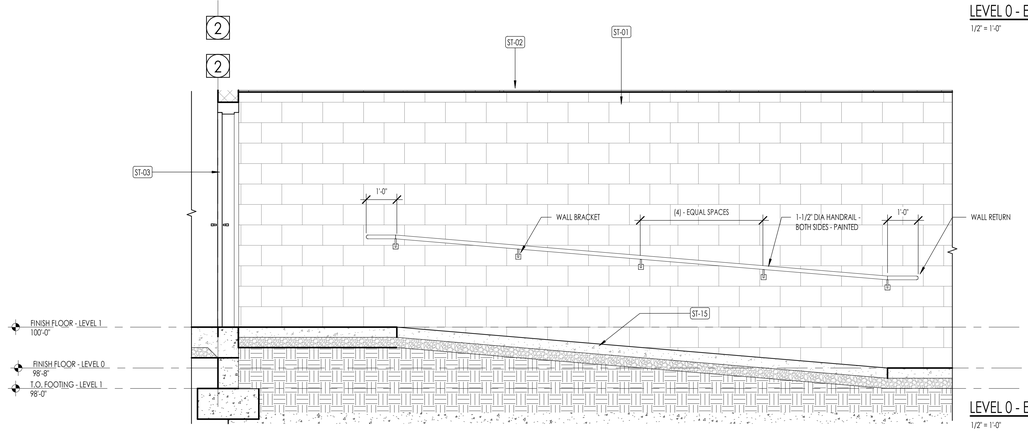
ENLARGED STAIR #3 LEVEL 2
1/2" = 1'-0"
1 A312



STAIR #3 SECTION 1
1/2" = 1'-0"
2 A312



LEVEL 0 - ENLARGED RAMP PLAN
1/2" = 1'-0"
3 A312



LEVEL 0 - ENLARGED RAMP SECTION
1/2" = 1'-0"
4 A312

STAIR PLAN GENERAL NOTES

STAIR PLAN KEYNOTES (ST)

KEYNOTES - STAIRS

ST-01	SCHEDULED WALL TYPE
ST-02	SCHEDULE CEILING
ST-03	SCHEDULED DOOR
ST-04	FLOOR STRUCTURE
ST-05	42" HIGH GUARDRAIL, ON OPEN SIDE OF STAIRS - PAINTED
ST-06	36" HIGH HANDRAIL, ON BOTH SIDES OF STAIRS - PAINTED
ST-07	HSS 12" X 3" X 1/4" STEEL STAIR STRINGER - PAINTED
ST-08	RETURN HANDRAIL TO FLOOR, GUARD OR WALL.
ST-09	40" (VIF) X 12" 11 GUAGE PRE-GALVANIZED STEEL TREAD W/ TRACTION-TREAD NON-SLIP SURFACE
ST-10	40" (VIF) X 12" 11 GUAGE PRE-GALVANIZED STEEL LANDING PLANKS W/ TRACTION-TREAD NON-SLIP SURFACE
ST-11	27" HIGH PROTECTION RAIL BENEATH STAIR
ST-12	PRE-GALVANIZED PERFORATED METAL RISER
ST-13	LIGHT FIXTURE
ST-14	SCHEDULE FINISH FLOOR
ST-15	STRUCTURAL CONCRETE FLOOR SLAB OVER 4" COMPACTED GRANULAR FILL
ST-16	STRUCTURAL COLUMN - PAINTED
ST-17	STRUCTURAL BEAM - PAINTED

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT:
OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

PROJECT NO. 22-111
DATE: MARCH 4, 2024

REVISIONS:

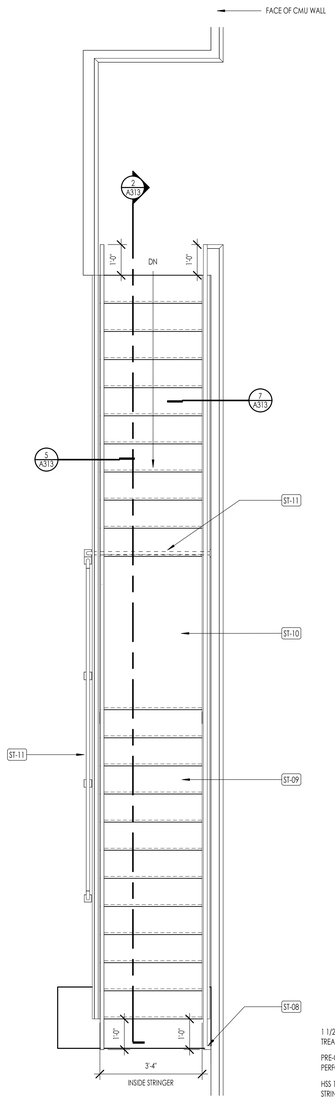
1 4.15.2024 ADDENDUM

BID SET - SECTION 1

SHEET TITLE:
ENLARGED STAIR PLANS/ SECTIONS

SHEET NUMBER:
A312

Plot Date: 4/15/2024 12:04:27 PM

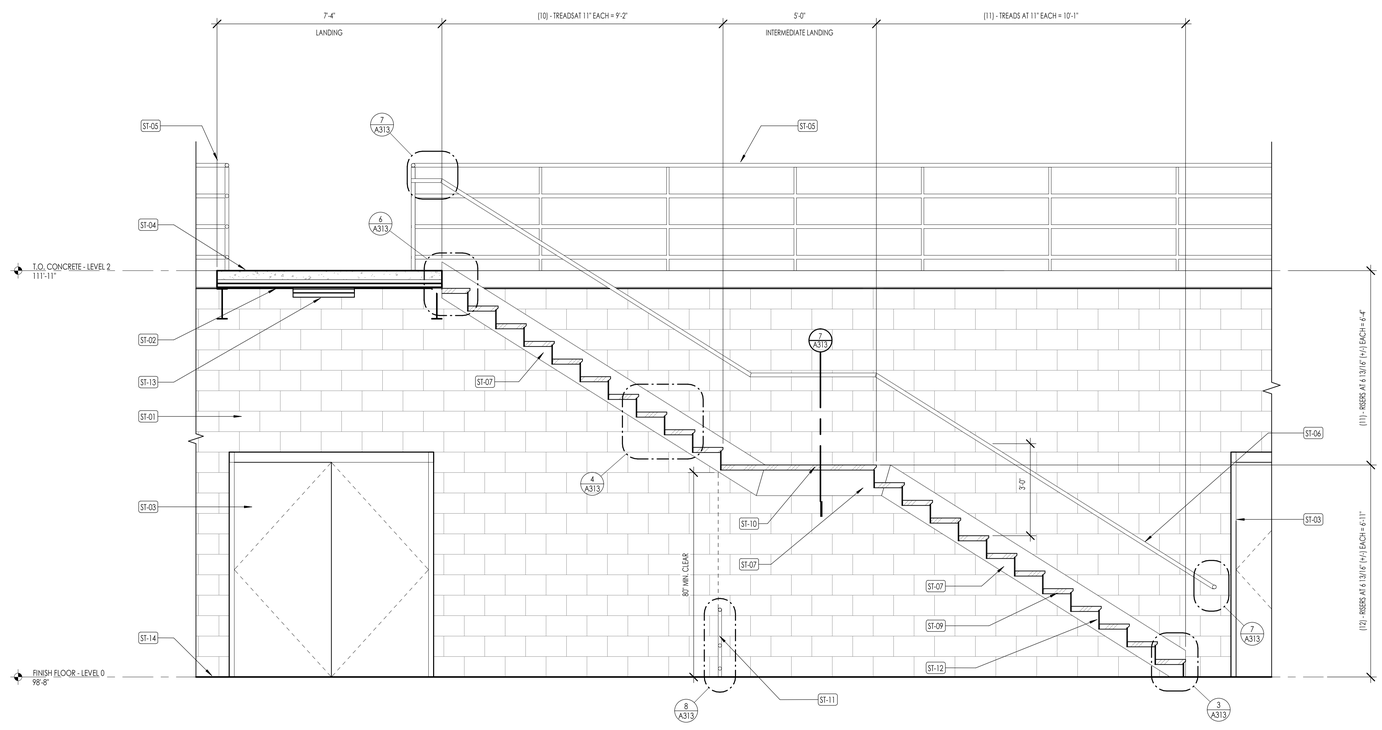


ENLARGED STAIR #4

1/2" = 1'-0"

1 A313

NORTH



STAIR SECTION STAIR #4

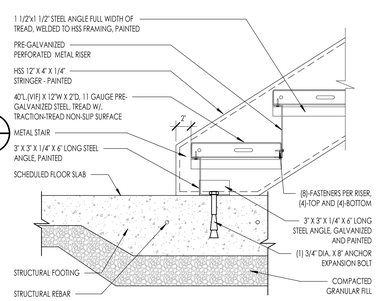
1/2" = 1'-0"

2 A313

STAIR DETAIL 4

1 1/2" = 1'-0"

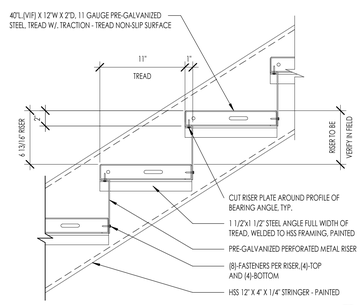
5 A313



STAIR DETAIL 1

1 1/2" = 1'-0"

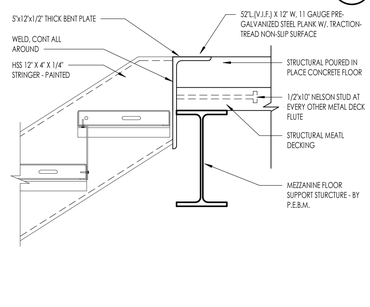
3 A313



STAIR DETAIL 2

1 1/2" = 1'-0"

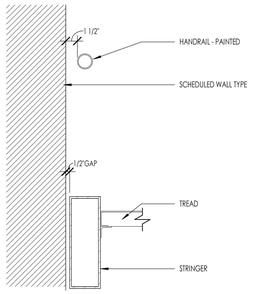
4 A313



STAIR DETAIL 3

1 1/2" = 1'-0"

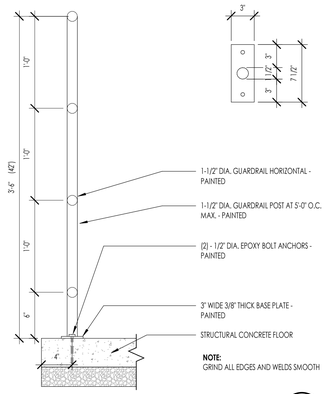
6 A313



STAIR DETAIL 6

1 1/2" = 1'-0"

7 A313



STAIR DETAIL 5

1 1/2" = 1'-0"

2018 IEC 1015.4 EXCEPTION 4

8 A313

STAIR PLAN GENERAL NOTES

STAIR PLAN KEYNOTES (ST)

KEYNOTES - STAIRS

ST-01	SCHEDULED WALL TYPE
ST-02	SCHEDULE CEILING
ST-03	SCHEDULED DOOR
ST-04	FLOOR STRUCTURE
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ST-06	36" HIGH HANDRAIL, ON BOTH SIDES OF STAIRS - PAINTED
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ST-12	PRE-GALVANIZED PERFORATED METAL RISER
ST-13	LIGHT FIXTURE
ST-14	SCHEDULE FINISH FLOOR

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

PROJECT NO. 22-111
DATE: MARCH 4, 2024

REVISIONS:

1 4.15.2024 ADDENDUM

BID SET - SECTION 1

SHEET TITLE:
ENLARGED STAIR PLANS/ SECTIONS

SHEET NUMBER:
A313

Plot Date: 4/15/2024 12:04:28 PM

DOOR SCHEDULE

MARK	DOOR SIZE WIDTH	DOOR SIZE HEIGHT	DOOR THICKNESS	DOOR MATERIAL	DOOR TYPE	DOOR FINISH	FRAME DETAILS HEAD	FRAME DETAILS JAMB	THRESHOLD	FRAME MATERIAL	FRAME TYPE	FRAME FINISH	FIRE RATING	HARDWARE GROUP #	REMARKS
~~100A~~	~~3'-1 1/2"~~	~~8'-5 1/2"~~	~~1 3/4"~~	~~GLASS~~	~~D9~~	~~CLEAR~~			~~13/A602~~	~~ALUMINUM~~	~~W3~~	~~ANODIZED~~	~~NON-RATED~~	~~MFG. 1~~	
101A	3'-0"	7'-0"	1 3/4"	GLASS-RATED	D10	CLEAR	1/A602	7/A602	-	HM	F4	PAINT	45 MIN.	11	RATED DOOR, FRAME AND GLAZING
101B	3'-0"	7'-0"	1 3/4"	WOOD	D6	STAIN	4/A602	20/A602	-	HM	F2	PAINT	45 MIN.	11	
101C	3'-0"	7'-0"	1 3/4"	METAL	D3	PAINT	4/A602	10/A602	-	HM	F1	PAINT	45 MIN.	14	
103A	6'-4"	7'-0"	1 3/4"	WOOD	D4	STAIN	14/A602	20/A602	-	HM	F2	PAINT	45 MIN.	5	
104A	3'-0"	7'-0"	3/4"	GLASS	D8	CLEAR	1/A602	7/A602	-	HM	F3	PAINT	NON-RATED	11	
104B	3'-0"	7'-0"	1 3/4"	WOOD	D6	PAINT	4/A602	10/A602	-	HM	F1	PAINT	45 MIN.	11	
105A	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	4	INSULATED
105B	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	5/A602	11/A602	-	METAL	-	-	NON-RATED	MFG. 6	INSULATED
105C	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	5/A602	11/A602	-	METAL	-	-	NON-RATED	MFG. 6	INSULATED
105D	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	5/A602	11/A602	-	METAL	-	-	NON-RATED	MFG. 6	INSULATED
105E	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	4	INSULATED
105F	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	4	INSULATED
105G	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	5/A602	11/A602	-	METAL	-	-	NON-RATED	MFG. 6	INSULATED
105H	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	5/A602	11/A602	-	METAL	-	-	NON-RATED	MFG. 6	INSULATED
105J	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	5/A602	11/A602	-	METAL	-	-	NON-RATED	MFG. 6	INSULATED
105K	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	4	INSULATED
106A	3'-8"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	4	INSULATED
106B	6'-4"	7'-0"	1 3/4"	METAL	D4	PAINT	2/A602	8/A602	-	HM	F1	PAINT	45 MIN.	5	
108A	6'-4"	7'-0"	1 3/4"	METAL	D4	PAINT	2/A602	8/A602	-	HM	F1	PAINT	45 MIN.	12	
109A	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	3	INSULATED
110A	8'-0"	8'-0"	2"	METAL	D13	PRE-FINISH	5/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
110B	2'-3"	2'-11"	1 3/4"	METAL	D11	PAINT	3/A602	9/A602	-	METAL	F5	PAINT	NON-RATED	4	
111A	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	2/A602	8/A602	-	HM	F1	PAINT	NON-RATED	13	
111B	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	2/A602	8/A602	-	HM	F1	PAINT	NON-RATED	13	
112A	3'-0"	7'-0"	1 3/4"	METAL	D3	PAINT	2/A602	8/A602	-	HM	F1	PAINT	45 MIN.	11	
112B	3'-0"	7'-0"	1 3/4"	METAL	D3	PAINT	2/A602	8/A602	-	HM	F1	PAINT	45 MIN.	11	
114A	3'-0"	7'-0"	1 3/4"	WOOD	D5	STAIN	14/A602	20/A602	12/A602	HM	F2	PAINT	NON-RATED	9	
115A	3'-0"	7'-0"	1 3/4"	WOOD	D5	STAIN	14/A602	20/A602	-	HM	F2	PAINT	NON-RATED	7	
117A	3'-0"	7'-0"	1 3/4"	WOOD	D5	STAIN	14/A602	20/A602	12/A602	HM	F2	PAINT	NON-RATED	9	
118A	2'-2"	7'-0"	1 3/4"	WOOD	D5	STAIN	14/A602	20/A602	12/A602	HM	F2	PAINT	NON-RATED	9	
119A	2'-2"	7'-0"	1 3/4"	WOOD	D5	STAIN	14/A602	20/A602	12/A602	HM	F2	PAINT	NON-RATED	9	
120A	2'-2"	7'-0"	1 3/4"	WOOD	D5	STAIN	14/A602	20/A602	12/A602	HM	F2	PAINT	NON-RATED	9	
123A	3'-0"	7'-0"	1 3/4"	WOOD	D5	STAIN	4/A602	10/A602	-	HM	F1	PAINT	NON-RATED	13	
124A	3'-0"	7'-0"	1 3/4"	WOOD	D5	STAIN	4/A602	10/A602	-	HM	F1	PAINT	NON-RATED	11	
125A	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	2/A602	8/A602	-	HM	F1	PAINT	NON-RATED	11	
126A	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	4	INSULATED
126B	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126C	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126D	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126E	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126F	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126G	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	4	INSULATED
126H	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	4	INSULATED
126J	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126K	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126L	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126M	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126N	14'-0"	14'-0"	2"	METAL	D12	PRE-FINISH	6/A602	11/A602	-	METAL	-	PAINT	NON-RATED	MFG. 6	INSULATED
126P	3'-0"	7'-0"	1 3/4"	METAL	D1	PAINT	3/A602	9/A602	11/A602	HM	F1	PAINT	NON-RATED	4	INSULATED
127A	6'-4"	7'-0"	1 3/4"	METAL	D4	PAINT	2/A602	8/A602	-	HM	F1	PAINT	45 MIN.	5	
128A	6'-4"	7'-0"	1 3/4"	METAL	D4	PAINT	2/A602	8/A602	-	HM	F1	PAINT	45 MIN.	5	
200A	3'-0"	7'-0"	1 3/4"	METAL	D3	PAINT	14/A602	20/A602	-	HM	F2	PAINT	45 MIN.	11	
202A	3'-0"	7'-0"	1 3/4"	METAL	D3	PAINT	14/A602	20/A602	-	HM	F2	PAINT	45 MIN.	7	
203A	3'-0"	7'-0"	1 3/4"	METAL	D3	PAINT	14/A602	20/A602	-	HM	F2	PAINT	45 MIN.	10	
204A	3'-0"	7'-0"	1 3/4"	METAL	D3	PAINT	14/A602	20/A602	-	HM	F2	PAINT	45 MIN.	10	
205A	6'-4"	7'-0"	1 3/4"	METAL	D4	PAINT	14/A602	20/A602	-	HM	F2	PAINT	45 MIN.	5	

DOOR SCHEDULE GENERAL NOTES

1. SEE SHEET A601 FOR DOOR AND FRAME TYPES.
2. CONTRACTOR SHALL FIELD VERIFY ALL DOOR OPENINGS PRIOR TO ORDERING ALL DOORS.
3. CONTRACTOR SHALL SUBMIT COMPLETE DOOR AND HARDWARE SHOP DRAWINGS AND SUBMITTALS FOR APPROVAL FOR EACH BUILDING PRIOR TO ORDERING AND TAKING RECEIPT OF DOOR ORDER. ARCHITECT SHALL REVIEW ALL DOORS FOR COMPLIANCE SPECIFICATIONS AND BUILDING CODE.
4. ALL DOORS REQUIRED TO BE RATED SHALL HAVE APPROPRIATE U.L. RATING AS INDICATED IN DOOR SCHEDULE AND SPECIFICATION. ALL DOORS SHALL HAVE LABEL ON DOOR AND FRAME FOR INSPECTION ON SITE. AND SHALL NOT BE REMOVED.
5. ALL DOORS SHALL BE INSTALLED SO AS NOT TO HAVE MORE THAN 1/2" THRESHOLD AT EACH DOOR.
6. SEE SPECS FOR HARDWARE SCHEDULE.
7. REFER TO INTERIOR DESIGNER FOR CORRECT DOOR STYLES, SPECIES, AND FINISHES.
8. OPENINGS BETWEEN GARAGE AND RESIDENCE SHALL BE EQUIPPED WITH SOLID WOOD DOOR NOT LESS THAN 1 3/8 INCHES THICK. SOLID HONEYCOMB CORE STEEL DOORS NOT LESS THAN 1 3/8 INCHES THICK, OR 20 MINUTE FIRE RATED DOORS. SEE I.R.C. SECTION R302.5.1.

DOOR GLAZING

1	1/4" GLASS
2	1" INSULATED GLASS
3	INSULATED GALSS
T	TEMPERED

HARDWARE GROUPS

H1 MECHANICAL ROOM
- PAIR SPRING HINGES
- SMOKE SEAL
- 1 PASSAGE SET

H2 OVERHEAD GARAGE DOORS
- GARAGE ENTRY PROVIDED BY OVERHEAD DOOR MANUFACTURER

H3 SECONDARY ENTRY DOOR
- 3 PAIR SPRING HINGES
- 1 SMOKE SEAL
- 1 LOCKSET
- 1 DEADBOLT
- 1 THRESHOLD

H4 GARAGE/ HOUSE
- 3 PAIR HINGES
- 1 WEATHER STRIP
- 1 LOCKSET
- 1 DEADBOLT

H5 INTERIOR DOOR
- 3 PAIR HINGES
- 1 PASSAGE SET

H6 INTERIOR BARN DOOR
-

H7 INTERIOR DOUBLE BARN DOOR
-

H8 INTERIOR DOOR
- 3 PAIR HINGES
- 1 PRIVACY SET

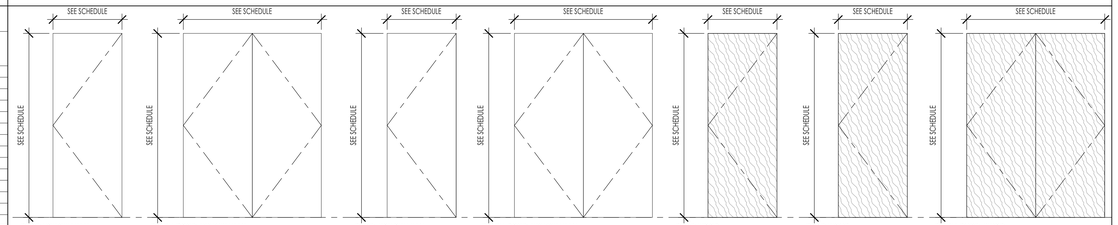
H9 INTERIOR DOUBLE DOOR
- 6 PAIR HINGES
- 2 KNOBS (SINGLE SIDE ONLY)

H10 ENTRY DOOR
- 1 WEATHER STRIP
- 1 THRESHOLD
- 1 LOCKSET
- 1 DEADBOLT
- 1 PIVOT HINGE (TOP AND BOTTOM)

H11 INTERIOR DOUBLE DOOR
- 6 PAIR HINGES
- 2 PASAGE SET

H12 POCKET DOOR
-

DOOR TYPES



DOOR: HOLLOW METAL
NON-RATED
HOLLOW METAL - F1 OR F2 FRAME
1 3/4" THICK
TYPE D1

DOUBLE DOOR: HOLLOW METAL
NON-RATED
HOLLOW METAL - F1 OR F2 FRAME
1 3/4" THICK
TYPE D2

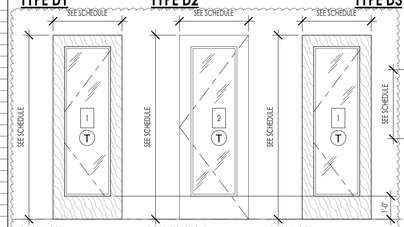
DOOR: HOLLOW METAL
45 MINUTE RATED
HOLLOW METAL - F1 OR F2 FRAME
1 3/4" THICK
TYPE D3

DOUBLE DOOR: HOLLOW METAL
45 MINUTE RATED
HOLLOW METAL - F1 OR F2 FRAME
1 3/4" THICK
TYPE D4

DOOR: WOOD
NON-RATED
HOLLOW METAL - F1 OR F2 FRAME
1 3/4" THICK
TYPE D5

DOOR: WOOD
45 MINUTE RATED
HOLLOW METAL - F1 OR F2 FRAME
1 3/4" THICK
TYPE D6

DOUBLE DOOR: WOOD
45 MINUTE RATED
HOLLOW METAL - F1 OR F2 FRAME
1 3/4" THICK
TYPE D7

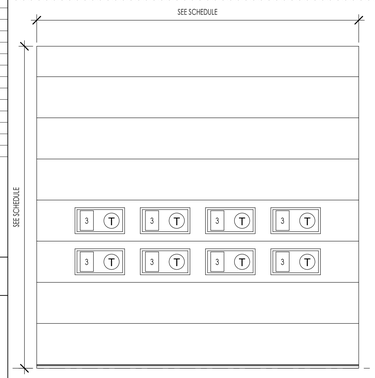


DOOR: WOOD
NON-RATED
WOOD - F3 FRAME
1 3/4" THICK
TYPE D8

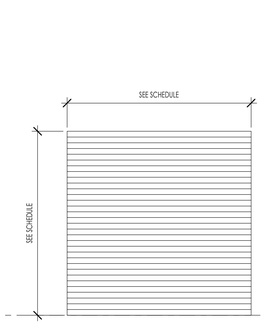
DOOR: STORERFRONT
NON-RATED
ALUMINUM - W3 FRAME
1 3/4" THICK
TYPE D9

DOOR: WOOD
45 MINUTE RATED
WOOD - F4 FRAME
1 3/4" THICK
TYPE D10

DOOR: HOLLOW METAL
NON-RATED
HOLLOW METAL - F5 FRAME
1 3/4" THICK
TYPE D11

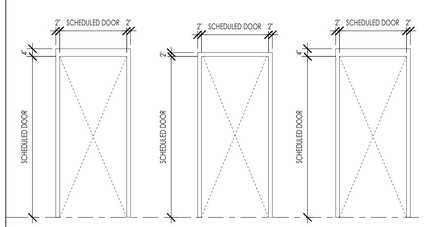


DOOR: GARAGE
INSULATED
2" THICK
TYPE D12



DOOR: GARAGE
COIL ROLL UP
2" THICK
TYPE D13

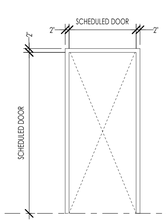
FRAME TYPES



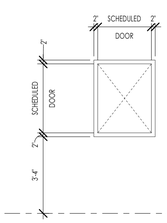
HOLLOW METAL - LOCATED IN CMU WALL
MAY BE SINGLE OR DOUBLE DOOR
PAINTED FINISH
TYPE F1

HOLLOW METAL - NON CMU WALL
MAY BE SINGLE OR DOUBLE DOOR
PAINTED FINISH
TYPE F2

HOLLOW METAL - NON CMU WALL
MAY BE SINGLE OR DOUBLE DOOR
PAINTED FINISH
TYPE F3



HOLLOW METAL - NON CMU WALL
45 MINUTE FIRE RATED
SINGLE DOOR - PAINTED FINISH
TYPE F4



HOLLOW METAL - LOCATED IN CMU WALL
PAINTED FINISH
TYPE F5

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Architecture
Interior Design
Landscape Architecture
Land Planning
Construction Management

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT:
OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

PROJECT NO. 22-111
DATE: MARCH 4, 2024

REVISIONS:

1 4.15.2024 ADDENDUM

BID SET - SECTION 1

SHEET TITLE:
DOOR SCHEDULE & ELEVATIONS

SHEET NUMBER:
A601

Plot Date: 4/15/2024 12:04:28 PM

d. Use E-70 XX (58 ksi yield, 70 ksi tensile) unless noted otherwise. E60 XX may be used for welding steel decks.
e. All intersecting steel shapes which are not bolted shall be connected by a fillet weld all around, unless noted otherwise. Where fillet weld sizes are not shown they shall be 1/16" less than the thinnest of the connected parts for thicknesses 1/4" and larger. Fillet welds on plates less than 1/4" shall be of the same size as the thinnest of the connected part.
f. Reinforcing Bars: Do not weld rebar. Do not substitute reinforcing bars for deformed bar anchors (DBAs), machine bolts, or headed stud anchors (HSAs).
g. Do not weld anchor bolts, including "tack" welds.
h. Headed Stud Anchors (HSAs) welding and deformed bar anchor welding shall conform to the manufacturer's specifications.

4. Bolted Connections:
   a. Use bolts for steel-to-steel connections, as noted herein or as noted on the drawings. Bolts shall be used in connections for simple span framing and beam (or girder) to bearing plate connections. Tighten bolts to a snug tight condition. See bolted connections schedule in drawings. Use hardened washers beneath the turned element of all bolts or nuts. Use hardened beveled washers, to compensate for the lack of parallelism, where the outer face of the bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolt axis. At oversized holes hardened washers or plates shall conform with ASTM F-436 and shall completely cover the slot after installation.
   b. Where a steel-to-steel beam connection is not shown, provide a standard AISC framed connection for one half the total uniform load capacity of the beam for the span and steel specified.
   c. Bolts, nuts, and washers shall not be reused.

5. Provide baseplate anchor rod connections to concrete elements that correlate with ACI 117. Circular or square washers are acceptable:

ANCHOR ROD DIAMETER	HOLE DIAMETER	WASHER SIZE	WASHER THICKNESS (MIN)
3/4"	1.5/16"	2"	1/4"
7/8"	1.9/16"	2.1/2"	5/16"
1"	1.7/8"	3"	3/8"
1.1/4"	2.1/8"	3.1/2"	1/2"
1.1/2"	2.3/8"	4"	1/2"
1.3/4"	2.7/8"	4.1/2"	5/8"
2"	3.1/4"	5"	3/4"
2.1/2"	3.3/4"	5.1/2"	7/8"

6. Provide full-depth web-stiffener plates at each side of all beams at all bearing points. Stiffener plates shall be the thickness called out below unless noted otherwise and shall be welded both sides with fillet welds all around:

FLANGE WIDTH	STIFFENER THICKNESS	WELD SIZE
Less than 8.1/4"	1/4"	3/16"
8.1/4" to 12.1/4"	3/8"	1/4"
12.1/4" to 16.1/2"	1/2"	5/16"
16.1/2" to 20.3/4"	5/8"	3/8"

COMPOSITE STEEL BEAMS

1. Headed Stud Anchors:
   a. Headed stud anchors shall 3/4" diameter and shall extend 1.1/2" above the top of the steel deck after welding. Headed stud anchors shall be welded through the metal deck to the top flange of the steel section or welded directly to the steel section.
   b. Anchors for composite steel beams are marked on the framing plans as "(x)". Locate anchors on composite steel beams (or segments of a beam) based on the following criteria:
      i. The number inside the brackets indicates the quantity of headed studs for the beam or segment of beam. When only one set of brackets is indicated, the headed studs shall be spaced uniformly over the entire beam length. For multiple sets of brackets, the headed studs shall be spaced uniformly over the segment of a beam associated with that set of brackets. Beam segments are delineated by adjoining framing members.
      ii. For steel beams perpendicular to deck corrugations: Place anchors in bottom of deck flutes. Where the number of uniformly placed anchors requires more than 1 anchor per deck corrugation, the first row shall be a complete uniform row with the remainder of the anchors placed in a second row equally at each end of the beam/segment.
      iii. For steel beams parallel to deck corrugations: Place anchors uniformly along beam the entire beam length/segment. The minimum center-to-center spacing of anchors shall be 4.1/2". Where the number of uniformly placed anchors is spaced at less than 4.1/2" o.c., the first row shall be a complete uniform row at 4.1/2" o.c. with the remainder of the anchors placed in a second row equally at each end of the beam/segment.
      iv. Anchors shall be placed in a single row within 1/4" of the center of the steel beam web. Where two rows of anchors are required, provide 3" center-to-center spacing between rows of anchors transverse to the steel beam web.
      v. The maximum center to center spacing shall not exceed 36".

2. Camber:
   a. Specific composite beams shall be precambered at the mid span. c= x" on the plans denotes pre-camber dimension (upward) in inches.
   b. Where a camber is not indicated at a steel beam, assume that c=0".

OPEN WEB STEEL JOISTS AND GIRDERS

1. All open web steel joist and girders shall be fabricated and erected in accordance with the latest edition of Steel Joist Institute (SJI), "Standard Specifications and Code of Standard Practice".

2. At the completion of fabrication, the steel joist manufacturer shall submit to the building official a certificate of compliance in accordance with IBC Section 1704.2.5 stating if the work was performed in accordance with approved construction documents and with SJI standard specifications.

3. Joists or girders with slopes greater than 1/2" per foot shall be designed to meet or exceed the load capacities, listed in the SJI load tables, of the joist or girder sizes indicated on the framing plan, as if the joists or girders were installed level.

4. Provide special bearing ends to accommodate slopes from sloped joists, sloped girders, or sloped bearing conditions.

5. Modifications to any joist or girder, including holes through the top and bottom chords, without the written consent and direction from the manufacturer are not allowed.

6. Joist and girder loads called out in the drawings are allowable stress design (ASD) loads.

7. Open web joist deflection shall be limited to L/180 for total loads and L/240 for roof live loads (or snow loads), unless noted otherwise on plans. The SJI required camber can be subtracted when considering the total load deflection requirements.

8. Camber joist and girder per typical SJI requirements, unless noted otherwise on plans.

9. Joist bridging shown on plans is for schematic purposes only; actual size, quantity and location of bridging shall be determined by the joist supplier per SJI. Coordinate bridging locations to avoid interference with mechanical, electrical and fire protection equipment and skylights.

METAL DECKING

1. Steel deck shall comply with the latest requirements of the Steel Deck Institute.

2. All deck shall be 3-span continuous minimum. In areas where 3-span conditions are not possible, the contractor shall provide heavier gage deck as required to provide the equivalent loading of the deck under a three-span condition.

3. Steel roof deck shall not be used to support loads from plumbing, HVAC ducts, light fixtures, architectural elements, or equipment of any kind, unless specifically noted. Light weight suspended acoustical ceilings with a total weight of 50 lbs per attachment may be hung from roof deck. The hangers shall be staggered to distribute the loads over multiple deck flutes.

4. Conduits are permitted in composite deck slabs subject to local code requirements and fire rating considerations. Conduits other than electrical or communication conduits shall not be permitted. When conduits are installed in the slab, the diameter shall be the lesser of 1" or 1/3 times the depth of concrete cover over the metal deck.
   a. No crossover of conduits shall occur.
   b. Conduit shall be spaced a minimum of 18" apart.
   c. The minimum clearance between conduit and the metal deck shall be 1".

5. All deck supporting members shall be dry before welding.

6. Clinch seams before welding interlocking seams.

7. Where deck is to receive sprayed-on fire proofing, painted deck shall be coated with special paint that will allow the sprayed-on fire proofing to adhere to the painted deck

Steel Floor Deck
   a. Steel floor deck shall be 3" deep X 20 gage minimum phosphatized/painted composite type "W" deck with interlocking side seams with the following properties:

	20 Gage	19 Gage	18 Gage	16 Gage
Minimum S ($in^3/_{ft}$) =	0.528	0.652	0.768	0.966
Minimum I ($in^4/_{ft}$) =	0.907	1.067	1.213	1.516

   ~~Deck shall be galvanized (G60) when used below mechanical~~ equipment rooms.

   1

   b. Where a deck span exceeds 10', see plan, use 18 gage minimum.
   c. ~~Steel deck with 3.1/2" thick (6.1/2" overall) normal weight concrete~~ slab shall have a minimum allowable diaphragm shear capacity of 1,100 lbs/ft for a 10'-6" deck span
   d. Fasten deck to supporting framing members with powder-driven fasteners. Powder-driven fasteners shall be as indicated below based on the steel framing thicknesses:

Steel Framing Thickness	Fastener	ICC-ESR or IAPMO report number
0.125" to 0.375"	Hilti X-HSN-24	ICC-ESR 2776
0.25" and up	Hilti X-ENP-19 L15	ICC-ESR 2776
0.113" to 0.155"	Pneutek SDK61075	ICC-ESR 2941
0.155" to 0.250"	Pneutek SDK63075	ICC-ESR 2941
0.188" to 0.312"	Pneutek K64062	ICC-ESR 2941
0.281" and up	Pneutek K66062	ICC-ESR 2941

   e. Fasteners shall be placed at the following spacings (Closer spacings may be used to develop minimum shear requirements):
      vi. 12" o.c. to supports perpendicular to deck corrugations (4 fasteners per 36" wide sheet).
      vii. 12" o.c. to all supports parallel to deck corrugations.
   f. In lieu or mechanical fasteners, contractor may weld deck to supporting framing members with 3/4" diameter puddle welds at the same spacing for deck pins as indicated above.
   g. Attach interlocking seams with one of the following.
      i. 1.1/2" long top seam welds at 4" o.c. maximum
      ii. Verco PunchLok II System at 4" o.c. maximum
      iii. ASC Delta Grip System at 4" o.c maximum
      Closer spacing may be used to develop minimum shear requirements.
   h. Provide a 2" minimum bearing at supports.

Steel Roof Deck
   i. Steel roof deck shall be 1.1/2" deep X 18 gage minimum painted, type "B" wide rib deck with interlocking side seams with the following properties:

	18 Gage	16 Gage
Minimum S ($in^3/_{ft}$) =	0.331	0.410
Minimum I ($in^4/_{ft}$) =	0.306	0.381

   j. Using 16 gage minimum in areas of snowdrift, see plan.

   1

   k. Minimum allowable deck diaphragm shear values shall be 900 lbs/ft for a 8'-0" deck span.
   l. Fasten deck to supporting framing members with powder-driven fasteners. Powder-driven fasteners shall be as indicated below based on the steel framing thicknesses:

Steel Framing Thickness	Fastener	ICC-ESR or IAPMO report number
0.125" to 0.375"	Hilti X-HSN-24	ICC-ESR 2776
0.25" and up	Hilti X-ENP-19 L15	ICC-ESR 2776
0.113" to 0.155"	Pneutek SDK61075	ICC-ESR 2941
0.155" to 0.250"	Pneutek SDK63075	ICC-ESR 2941
0.188" to 0.312"	Pneutek K64062	ICC-ESR 2941
0.281" and up	Pneutek K66062	ICC-ESR 2941

   m. Fasteners shall be placed at the following spacings (Closer spacings may be used to develop minimum shear requirements):
      i. 6" o.c. to all supports perpendicular to deck corrugations (7 fasteners per 36" sheet).
      ii. 6" o.c. to all supports parallel to deck corrugations.
   n. In lieu or mechanical fasteners, contractor may weld deck to supporting framing members with 3/4" diameter puddle welds at the same spacing for deck pins as indicated above.
   o. Attach interlocking seams with one of the following:
      i. 1.1/2" long top seam welds at 6" o.c. maximum
      ii. Verco PunchLok II System at 6" o.c. maximum
      iii. ASC Delta Grip System at 6" o.c maximum
      Closer spacing may be used to develop minimum shear requirements. A standard button punch may **not** be used in place of Verco PunchLok or DeltaGrip.
   p. Provide a 2" minimum bearing and a 4" lap at the splice points.

COLD-FORMED STEEL

1. All cold-formed steel shall meet the requirements of "Specifications for the Design of Cold-Formed Steel Structural Members" by American Iron and Steel Institute (AISI).

2. All cold-formed steel connectors shall be provided by The Steel Network. If the contractor elects to substitute for another manufacturer, the contractor shall submit a revised connector list, prior to construction, that includes the following information:
   a. Specified connector indicated on these plans
   b. Requested substitution connector
   c. Allowable capacity of the requested substitution connector

3. Light Gauge Steel Framing:
   a. Galvanized steel shall meet the minimum requirements of ASTM A653 (Fy = 50 ksi) for 97 mil (12 gauge), 68 mil (14 gauge) and 54 mil (16 gauge). For 43 mil (18 gauge) and lighter galvanized steel shall meet and ASTM A653 (Fy = 33 ksi). Galvanized coatings must meet the ASTM A924.
   b. Follow all manufacturers' recommendations for the use of these products.
   c. Unless noted otherwise, all welded connections shall be done according to AWS standards.
   d. All interior non-bearing steel-stud walls that extend above the ceiling but do not attach to the structure above shall be brace with diagonal metal-stud braces (45 degrees). The kl/r ratio of the brace shall not exceed 200 and shall not be spaced further apart than 10'-0" o.c. Connect diagonal braces to the top of the steel stud walls and to the top flange of the steel beams with two #10 tek screws minimum. Where a concrete deck occurs above, use two powder-driven fasteners per diagonal brace. Other approved methods may be used.

4. Wood Sheathing
   a. Wood sheathing shall meet the minimum performance criteria given in APA PRP-108, Performance Standards and Policies for Structural-Use Panels, Form E445, Voluntary Product Standard PS 1 & PS 2 and Performance Standard for Wood-Based Structural-Use Panels, Form S350, and Structural Plywood, Form H860.

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT:
OPERATIONS BUILDING - SECTION 1
497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BHB PROJECT NO. 230074
DATE: APRIL 15, 2024
REVISIONS:
1 04/15/24 ADDENDUM 1

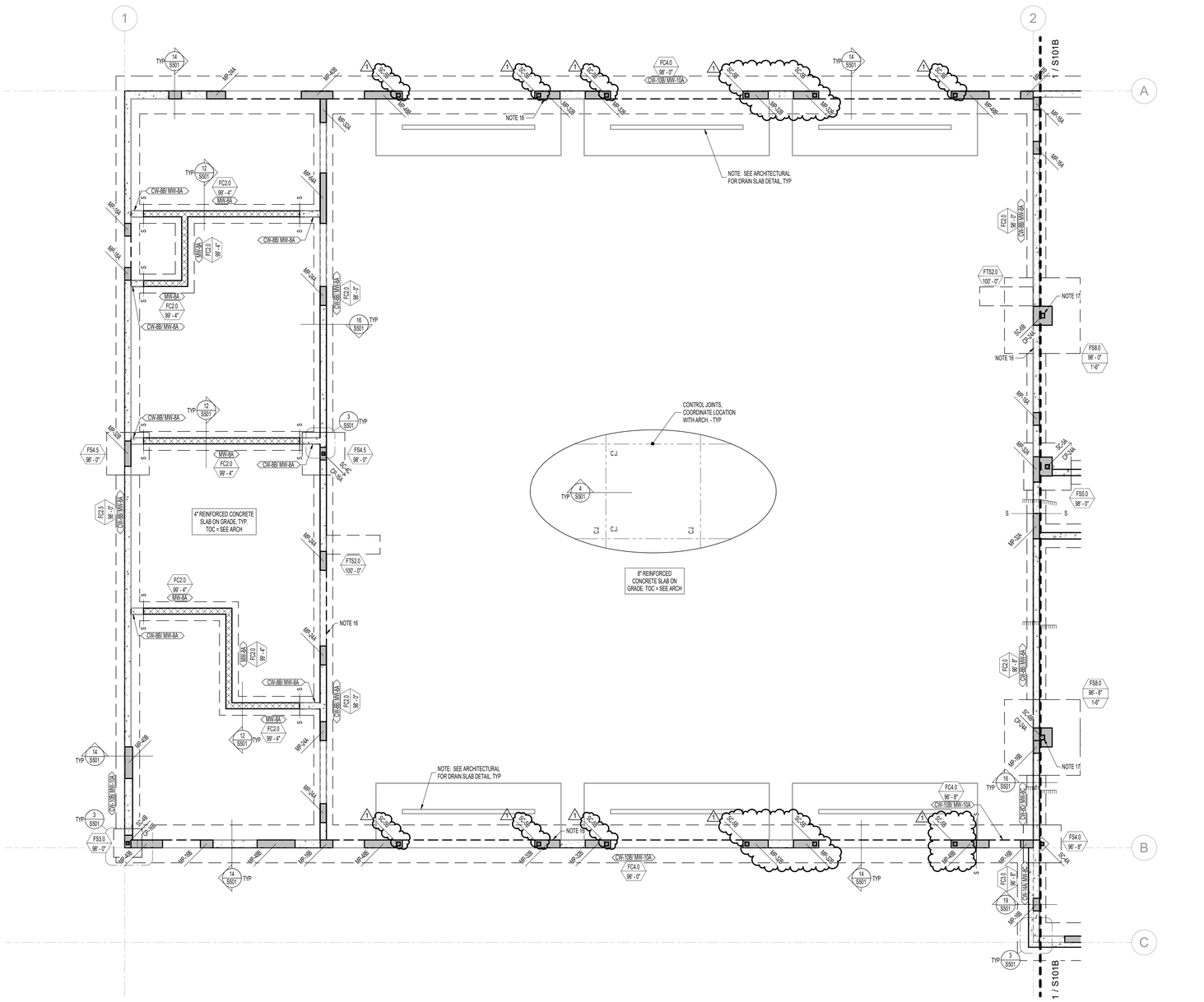
BID SET - SECTION 1

SHEET TITLE:
GENERAL STRUCTURAL NOTES

SHEET NUMBER:
S002

FOOTING AND FOUNDATION PLAN NOTES

1. COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
3. SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LOCATIONS.
4. SEE "EARTHWORK" NOTES ON SHEET S001 FOR MINIMUM F.L.L. REQUIRED BENEATH FOOTINGS.
5. ALL SPOT FOOTINGS SHALL BE CENTERED UNDER COLUMNS (UNO).
6. SEE DETAILS 1/S501 AND 2/S501 FOR CONDITION WHERE BURIED PIPES RUN PARALLEL AND PERPENDICULAR TO FOOTINGS.
7. SEE DETAIL 4/S501 FOR TYPICAL CONTROL/CONSTRUCTION JOINTS IN CONCRETE SLAB ON GRADE.
8. SEE DETAIL 5/S501 FOR CONTROL JOINTS AT SLAB DEPRESSIONS.
9. SEE DETAIL 6/S501 FOR SLAB REINFORCING WHERE CONTROL JOINTS ARE DISCONTINUOUS.
10. SEE DETAIL 7/S501 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
11. SEE DETAIL 8/S501 FOR CONDITION AT RECESSES IN MASONRY WALLS.
12. SEE DETAIL 9/S501 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
13. SEE DETAIL 10/S501 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
14. SEE DETAIL 13/S501 FOR ANCHORAGE OF HOUSEKEEPING PADS.
15. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.
16. PROVIDE POUR-BACK x CONTINUOUS STRIP AS REQ'D ALONG WALL LINE, SEE DETAIL 17/S501.
17. BLOCK OUT FOUNDATION WALL AS REQUIRED FOR INSTALLATION OF ANCHOR BOLTS AND BASE PLATE



1 FOOTING AND FOUNDATION PLAN - AREA A

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT:
OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BHB PROJECT NO. 230074
DATE: APRIL 15, 2024

REVISIONS:
1 04/15/24 ADDENDUM 1

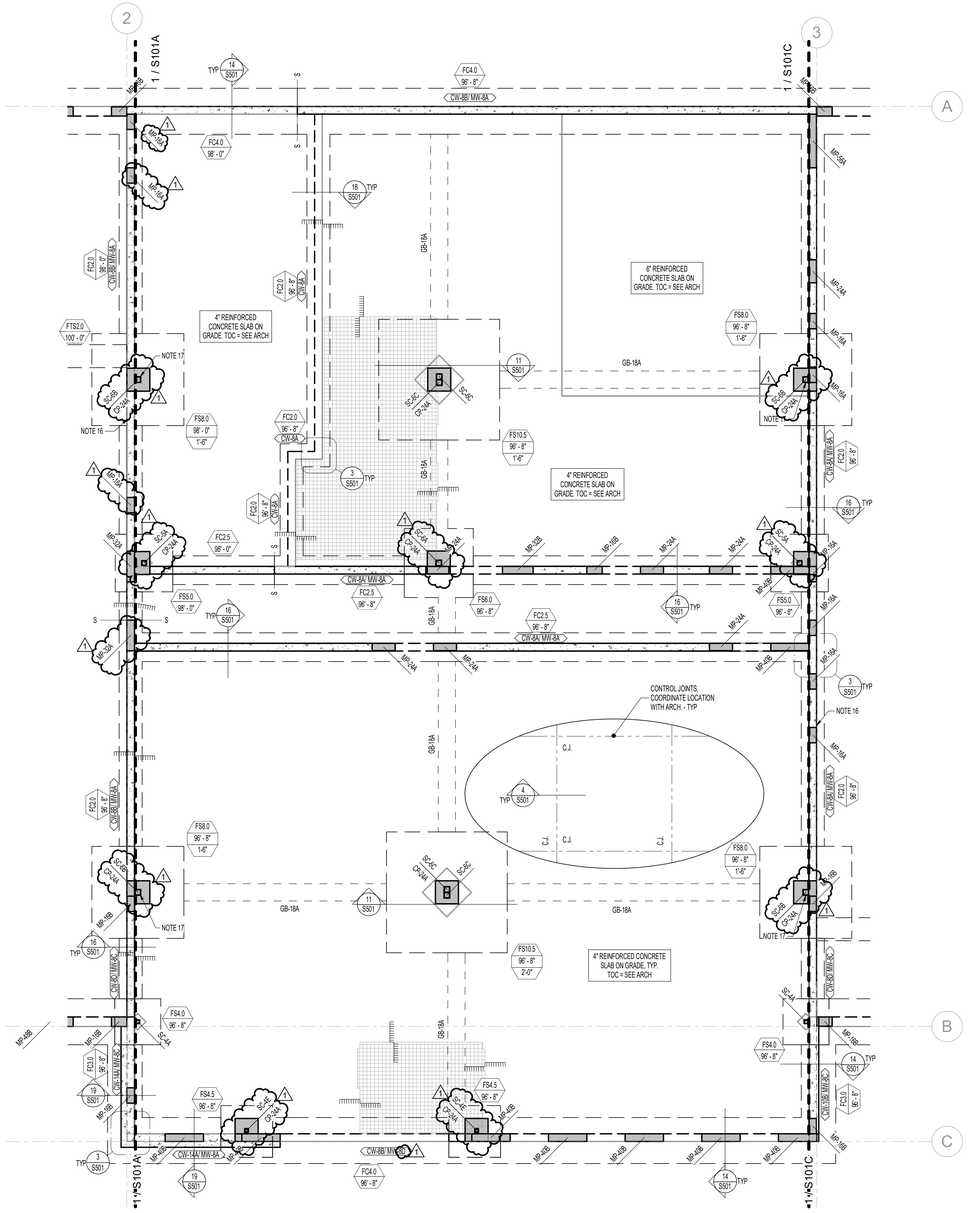
BID SET - SECTION 1

SHEET TITLE:
FOOTING AND FOUNDATION PLAN - AREA A

SHEET NUMBER:
S101A

FOOTING AND FOUNDATION PLAN NOTES

1. COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
3. SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LOCATIONS.
4. SEE "EARTHWORK" NOTES ON SHEET S001 FOR MINIMUM F.L.L. REQUIRED BENEATH FOOTINGS.
5. ALL SPOT FOOTINGS SHALL BE CENTERED UNDER COLUMNS (UNO).
6. SEE DETAILS 1/S501 AND 2/S501 FOR CONDITION WHERE BURIED PIPES RUN PARALLEL AND PERPENDICULAR TO FOOTINGS.
7. SEE DETAIL 4/S501 FOR TYPICAL CONTROL/CONSTRUCTION JOINTS IN CONCRETE SLAB ON GRADE.
8. SEE DETAIL 5/S501 FOR CONTROL JOINTS AT SLAB DEPRESSIONS.
9. SEE DETAIL 6/S501 FOR SLAB REINFORCING WHERE CONTROL JOINTS ARE DISCONTINUOUS.
10. SEE DETAIL 7/S501 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
11. SEE DETAIL 8/S501 FOR CONDITION AT RECESSES IN MASONRY WALLS.
12. SEE DETAIL 9/S501 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
13. SEE DETAIL 10/S501 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
14. SEE DETAIL 13/S501 FOR ANCHORAGE OF HOUSEKEEPING PADS.
15. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.
16. PROVIDE POUR-BACK x CONTINUOUS STRIP AS REQD ALONG WALL LINE. SEE DETAIL 17/S501.
17. BLOCK OUT FOUNDATION WALL AS REQUIRED FOR INSTALLATION OF ANCHOR BOLTS AND BASE PLATE



1 FOOTING AND FOUNDATION PLAN - AREA B

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BHB PROJECT NO. 230074
DATE: APRIL 15, 2024

REVISIONS:

1 04/15/24 ADDENDUM 1

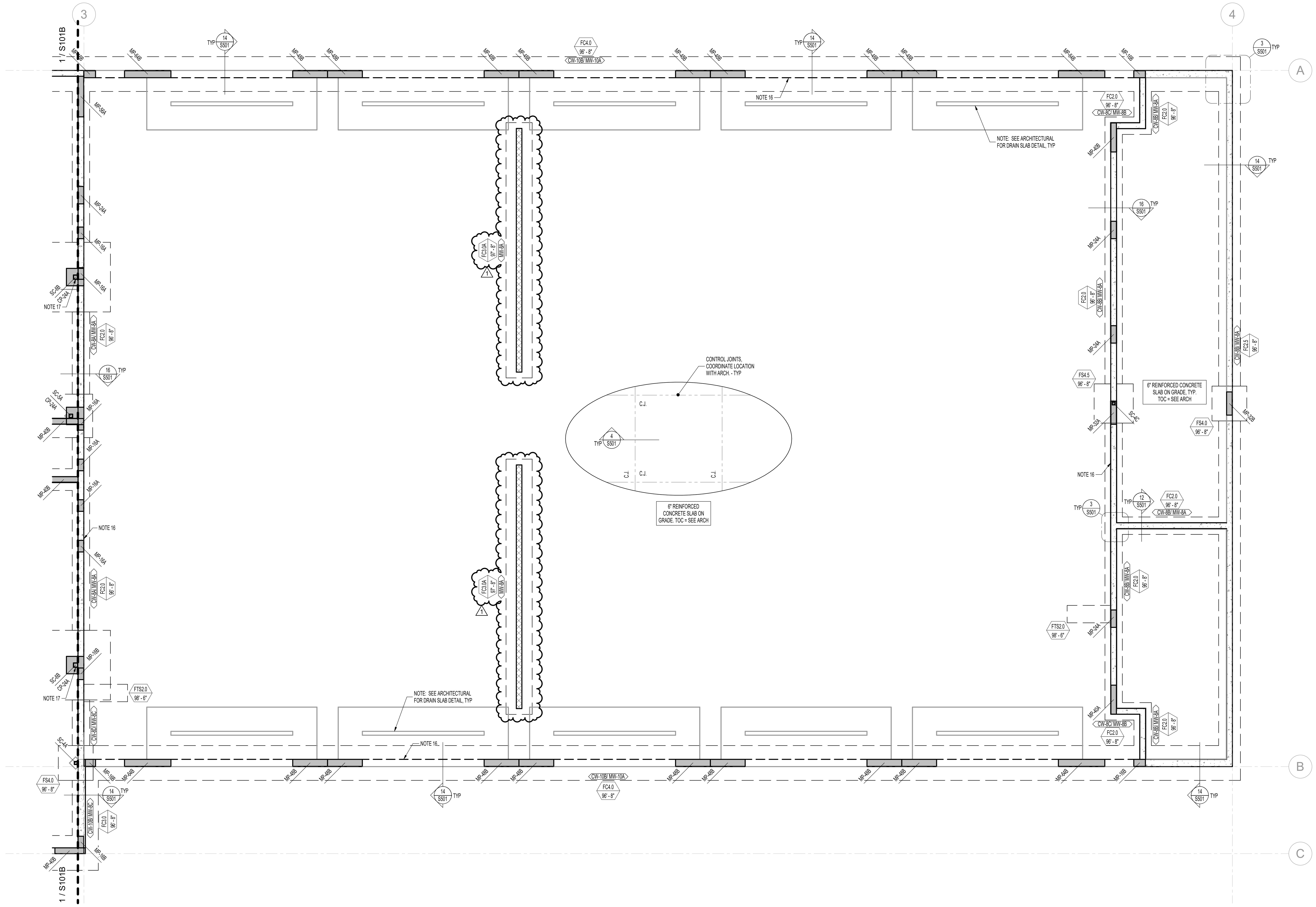
BID SET - SECTION 1

SHEET TITLE:
FOOTING AND FOUNDATION PLAN - AREA B

SHEET NUMBER:
S101B

FOOTING AND FOUNDATION PLAN NOTES

1. COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
3. SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LOCATIONS.
4. SEE "EARTHWORK" NOTES ON SHEET S001 FOR MINIMUM FILL REQUIRED BENEATH FOOTINGS.
5. ALL SPOT FOOTINGS SHALL BE CENTERED UNDER COLUMNS (UNO).
6. SEE DETAILS 1/S501 AND 2/S501 FOR CONDITION WHERE BURIED PIPES RUN PARALLEL AND PERPENDICULAR TO FOOTINGS.
7. SEE DETAIL 4/S501 FOR TYPICAL CONTROL/CONSTRUCTION JOINTS IN CONCRETE SLAB ON GRADE.
8. SEE DETAIL 5/S501 FOR CONTROL JOINTS AT SLAB DEPRESSIONS.
9. SEE DETAIL 6/S501 FOR SLAB REINFORCING WHERE CONTROL JOINTS ARE DISCONTINUOUS.
10. SEE DETAIL 7/S501 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
11. SEE DETAIL 8/S501 FOR CONDITION AT RECESSES IN MASONRY WALLS.
12. SEE DETAIL 9/S501 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
13. SEE DETAIL 10/S501 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
14. SEE DETAIL 13/S501 FOR ANCHORAGE OF HOUSEKEEPING PADS.
15. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.
16. PROVIDE POUR-BACK x CONTINUOUS STRIP AS REQ'D ALONG WALL LINE, SEE DETAIL 17/S501.
17. BLOCK OUT FOUNDATION WALL AS REQUIRED FOR INSTALLATION OF ANCHOR BOLTS AND BASE PLATE



1 FOOTING AND FOUNDATION PLAN - AREA C

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

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BHB PROJECT NO. 230074
DATE: APRIL 15, 2024

REVISIONS:
1 04/15/24 ADDENDUM 1

BID SET - SECTION 1

SHEET TITLE:
FOOTING AND FOUNDATION PLAN - AREA C

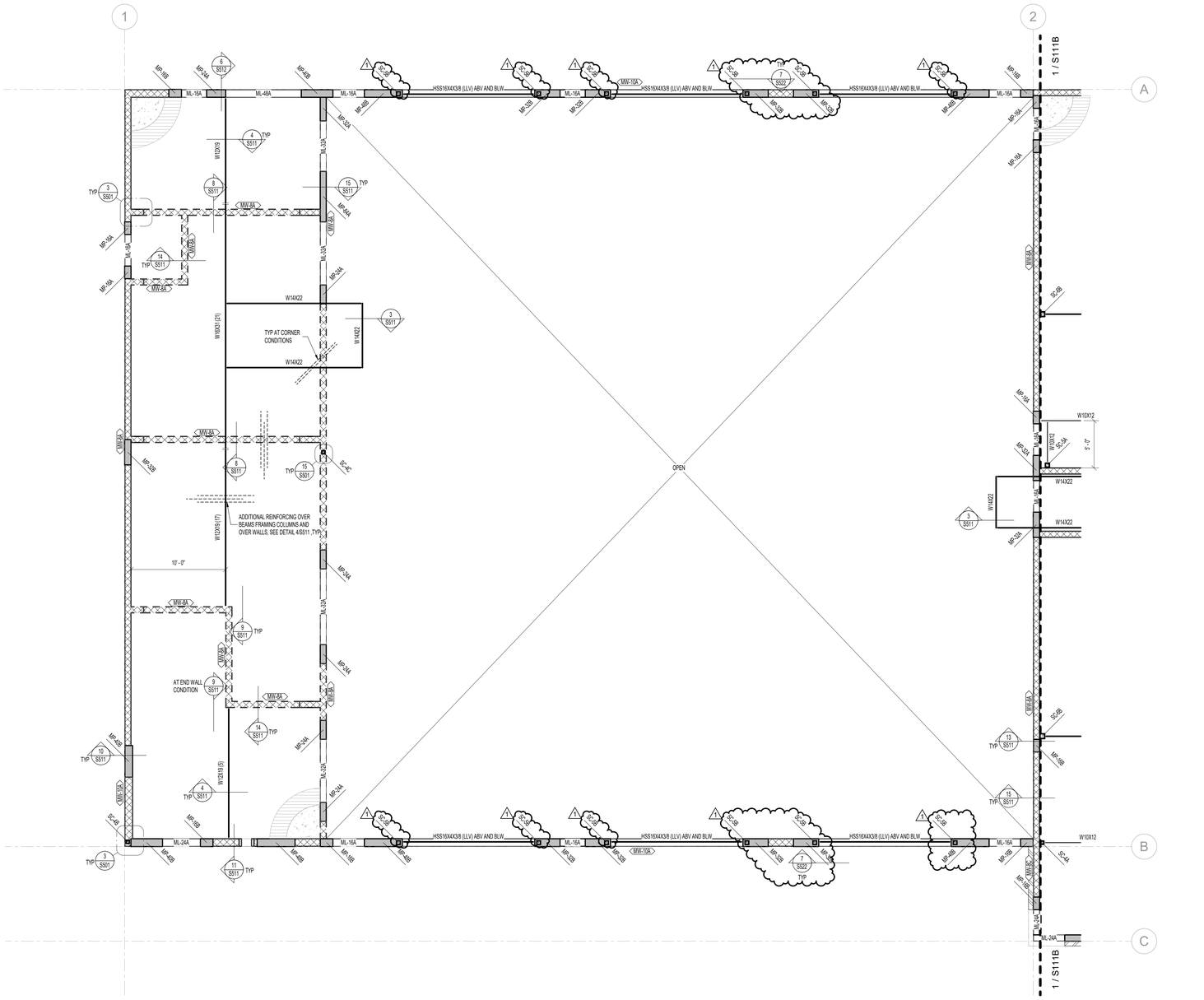
SHEET NUMBER:
S101C

FLOOR FRAMING DESIGN LOADS

FLOOR LOADS	
DEAD LOAD	75psf
LIVE LOAD	250psf
TOTAL LOAD	325psf

FLOOR FRAMING PLAN NOTES

1. VERIFY ALL FLOOR OPENINGS FOR MECHANICAL SHAFTS, STAIRS, ETC. WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. SEE DETAILS 1/S511 AND 2/S511 FOR FRAMING AROUND MISCELLANEOUS FLOOR OPENINGS.
3. COORDINATE SIZE AND LOCATION OF ALL MECHANICAL OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
4. CAMBER JOISTS FOR DEADLOAD DEFLECTION.
5. SEE DETAIL 7/S501 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
6. SEE DETAIL 8/S501 FOR CONDITION AT RECESSES IN MASONRY WALLS.
7. SEE DETAIL 9/S501 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
8. SEE DETAIL 10/S501 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
9. SEE DETAIL 13/S501 FOR ANCHORAGE OF HOUSEKEEPING PADS.
10. SEE DETAIL 7/S511 FOR STEEL BRACE CONNECTIONS AND LOCATIONS.
11. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.



1 LEVEL 2 FLOOR FRAMING PLAN - AREA A

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

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DATE: APRIL 15, 2024

REVISIONS:

1 04/15/24 ADDENDUM 1

BID SET - SECTION 1

SHEET TITLE:
LEVEL 2 FRAMING PLAN - AREA A

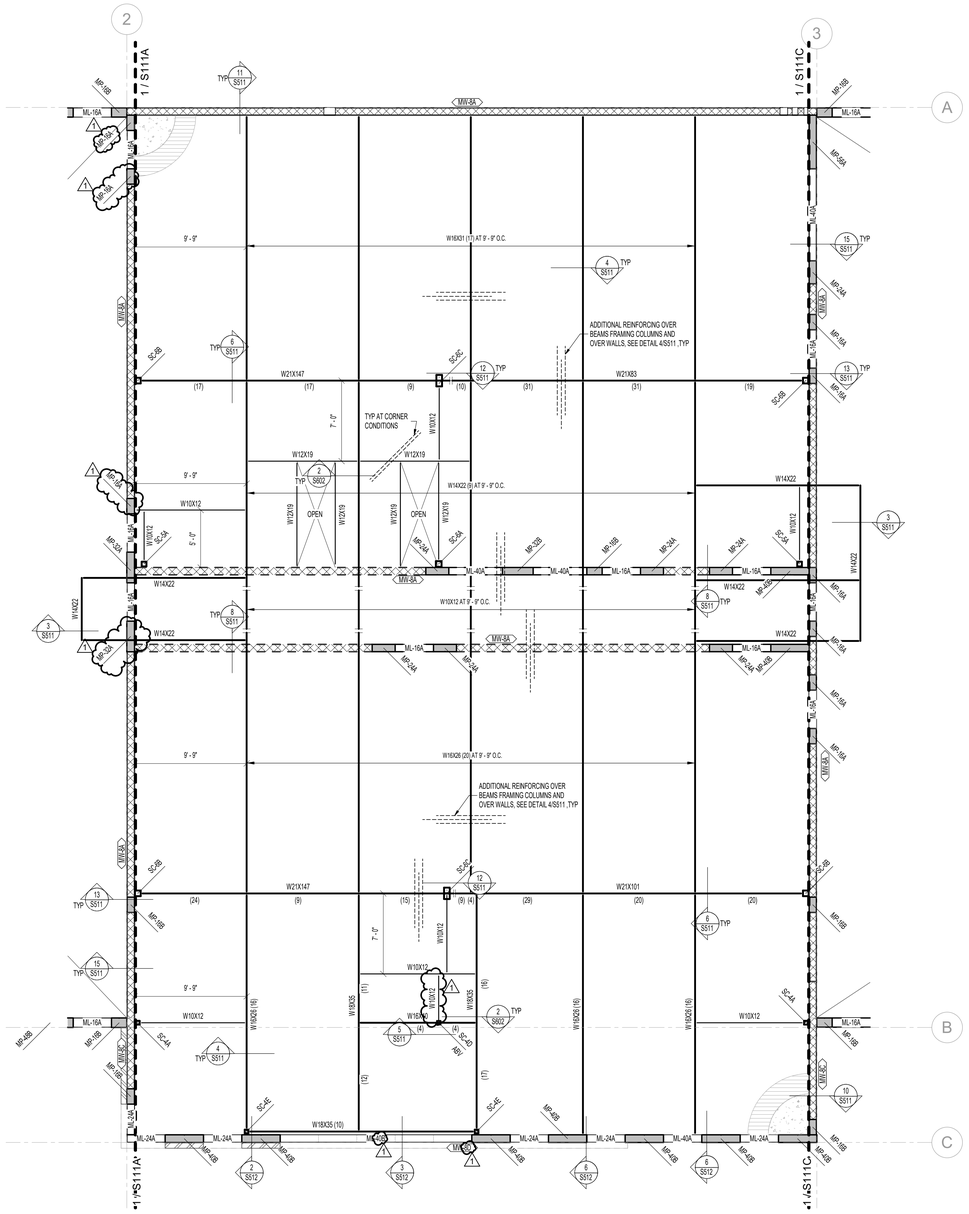
SHEET NUMBER:
S111A

FLOOR FRAMING DESIGN LOADS

FLOOR LOADS	
DEAD LOAD	75psf
LIVE LOAD	250psf
TOTAL LOAD	325psf

FLOOR FRAMING PLAN NOTES

1. VERIFY ALL FLOOR OPENINGS FOR MECHANICAL SHAFTS, STAIRS, ETC. WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. SEE DETAILS 1/S511 AND 2/S511 FOR FRAMING AROUND MISCELLANEOUS FLOOR OPENINGS.
3. COORDINATE SIZE AND LOCATION OF ALL MECHANICAL OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
4. CAMBER JOISTS FOR DEADLOAD DEFLECTION.
5. SEE DETAIL 7/S501 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
6. SEE DETAIL 8/S501 FOR CONDITION AT RECESSES IN MASONRY WALLS.
7. SEE DETAIL 9/S501 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
8. SEE DETAIL 10/S501 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
9. SEE DETAIL 13/S501 FOR ANCHORAGE OF HOUSEKEEPING PADS.
10. SEE DETAIL 7/S511 FOR STEEL BRACE CONNECTIONS AND LOCATIONS.
11. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.



1 LEVEL 2 FLOOR FRAMING PLAN - AREA B

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

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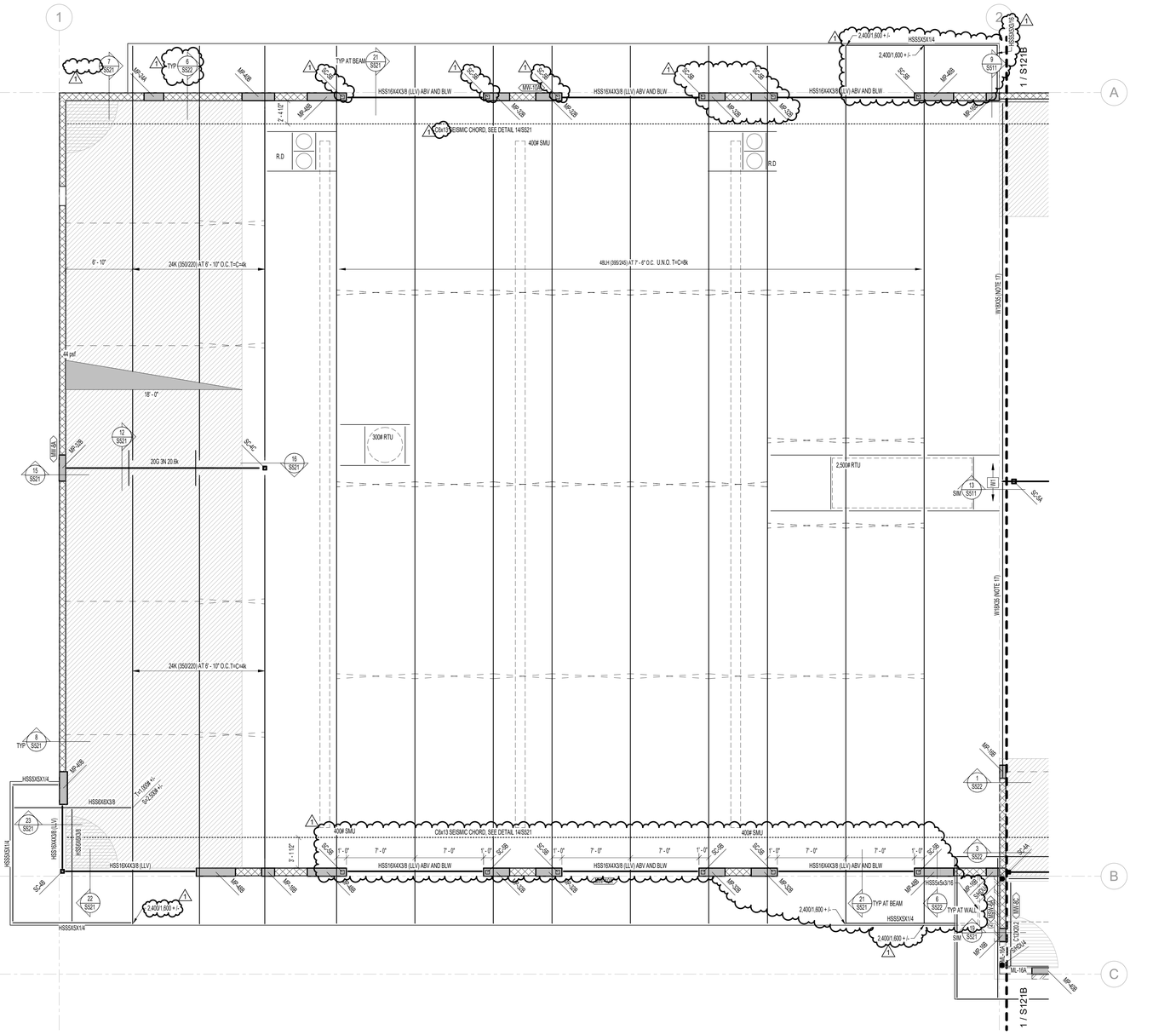
BID SET - SECTION 1

SHEET TITLE:
LEVEL 2 FRAMING PLAN - AREA B

SHEET NUMBER:
S111B



1 ROOF FRAMING PLAN - AREA A

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

ROOF FRAMING DESIGN LOADS

ROOF LOADS	
DEAD LOAD	25psf
SNOW LOAD	32psf + DRIFT
TOTAL LOAD	57psf

1. AT OVERHANGS, INCREASE SNOW LOADING BY A FACTOR OF 2.4.

ROOF FRAMING PLAN NOTES

1. VERIFY ALL ROOF OPENINGS FOR MECHANICAL SHAFTS, DRAINS, ETC. WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. JOIST SUPPLIER SHALL DESIGN ALL ROOF JOIST BEARING ENDS AT MASONRY WALLS TO TRANSFER 8,000# (ALLOWABLE) AXIAL LOAD FOR 10" CMU WALLS AND 4,000# (ALLOWABLE) AXIAL LOAD FOR 8" CMU WALLS THROUGH JOIST BEARING SHOE.
3. ALL JOISTS SHALL HAVE 5" DEEP BEARING ENDS (UNO).
4. ALL ROOF OPENINGS GREATER THAN, OR EQUAL TO, 12" x 12" SHALL BE FRAMED AS INDICATED IN DETAILS 1/SS21 AND 2/SS21. FOR OPENINGS WHICH CUT LESS THAN TWO DECK FLUTES, SEE DETAIL 3/SS21.
5. SEE DETAIL 4/SS21 WHEN CONCENTRATED LOADS ARE LOCATED MORE THAN 6" FROM JOIST OR JOIST GIRDER PANEL POINT.
6. SEE DETAIL 5/SS21 WHEN MECHANICAL UNITS ARE HUNG BELOW JOISTS.
7. VERIFY SIZE, WEIGHT, AND LOCATION OF ALL ROOF TOP MECHANICAL UNITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. SEE DETAIL 6/SS21 FOR STEEL FRAMES AT ALL ROOF TOP EQUIPMENT. COORDINATE OPENINGS WITH MECHANICAL, ELECTRICAL, AND GENERAL CONTRACTORS
8. LOCATE MISCELLANEOUS MECHANICAL OPENINGS BETWEEN JOISTS, NOT UNDERNEATH THEM.
9. OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION TO THE UNIFORM AND POINT LOADS SHOWN.
10. JOIST BRIDGING SHOWN ON PLANS IS FOR REPRESENTATION ONLY; ACTUAL SIZE, QUANTITY, AND LOCATION WILL BE DETERMINED BY THE JOIST SUPPLIER PER 'SJI' REQUIREMENTS. ALL BRIDGING AND BRIDGING ANCHORS NEED TO BE IN PLACE BEFORE APPLYING ANY LOADS. WHERE SKYLIGHT OR MECHANICAL UNITS/DUCTS INTERRUPT HORIZONTAL BRIDGING, PROVIDE CROSS BRIDGING AT JOIST SPACES ON EACH SIDE OF THE OPENING. WHERE DIAGONAL BRIDGING CONFLICTS WITH MECHANICAL DUCTS, REMOVE DIAGONAL BRIDGING AND REPLACE WITH HORIZONTAL BRIDGING AFTER ROOF DECK IS IN PLACE.
11. JOIST DESIGNER SHALL DESIGN JOISTS AND GIRDERS AND SUPPLY ADDITIONAL BRIDGING AS REQUIRED FOR UPLIFT DUE TO WIND. ASSUME:
    - 0.6DL + 12psf
    - 0.6WL + 25psf (UPLIFT)
    - 13psf NET UPLIFT (ASD)
    - NO 1/3 STRESS INCREASE ALLOWED.
12. SEE DETAIL 7/SS21 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
13. SEE DETAIL 8/SS21 FOR CONDITION AT RECESSES IN MASONRY WALLS.
14. SEE DETAIL 9/SS21 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
15. SEE DETAIL 10/SS21 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
16. SEE ARCHITECTURAL PLANS FOR DIMENSIONS TO ALL STEEL COLUMNS.
17. PROVIDE HSS5x5x3/16x(CONT BLOCKING OVER BEAM. BRACE BOTTOM FLANGE OF BEAM PER DETAIL 7/SS11 AT 6'-0" O.C. MAX. BEAM RUNS CONTINUOUS OVER MASONRY WALL.

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BHB PROJECT NO. 230074
DATE: APRIL 15, 2024

REVISIONS:
1 04/15/24 ADDENDUM 1

BID SET - SECTION 1

SHEET TITLE:
ROOF FRAMING PLAN - AREA A

SHEET NUMBER:
S121A

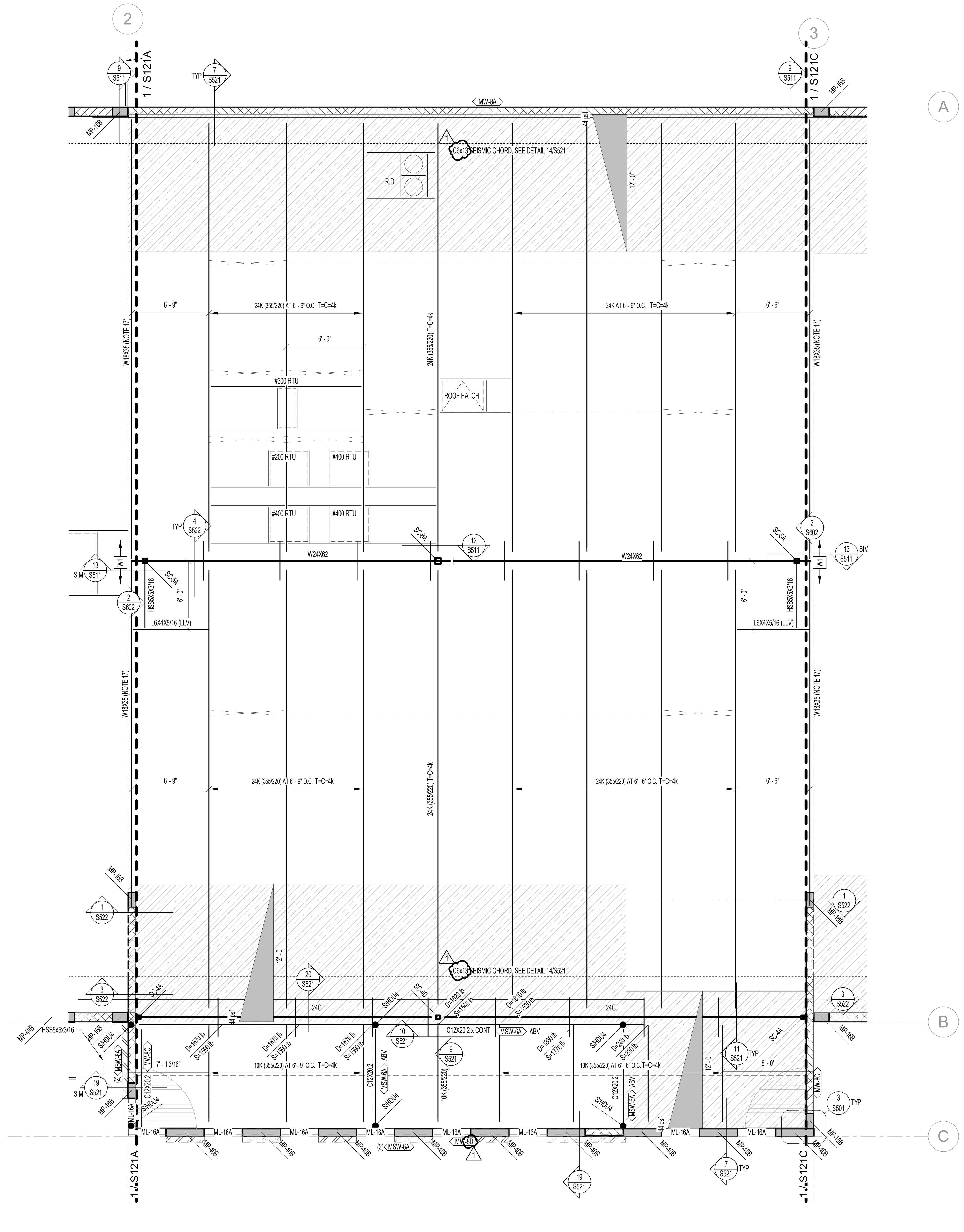
ROOF FRAMING DESIGN LOADS

ROOF LOADS	
DEAD LOAD	25psf
SNOW LOAD	32psf + DRIFT
TOTAL LOAD	57psf

1. AT OVERHANGS, INCREASE SNOW LOADING BY A FACTOR OF 2.4.

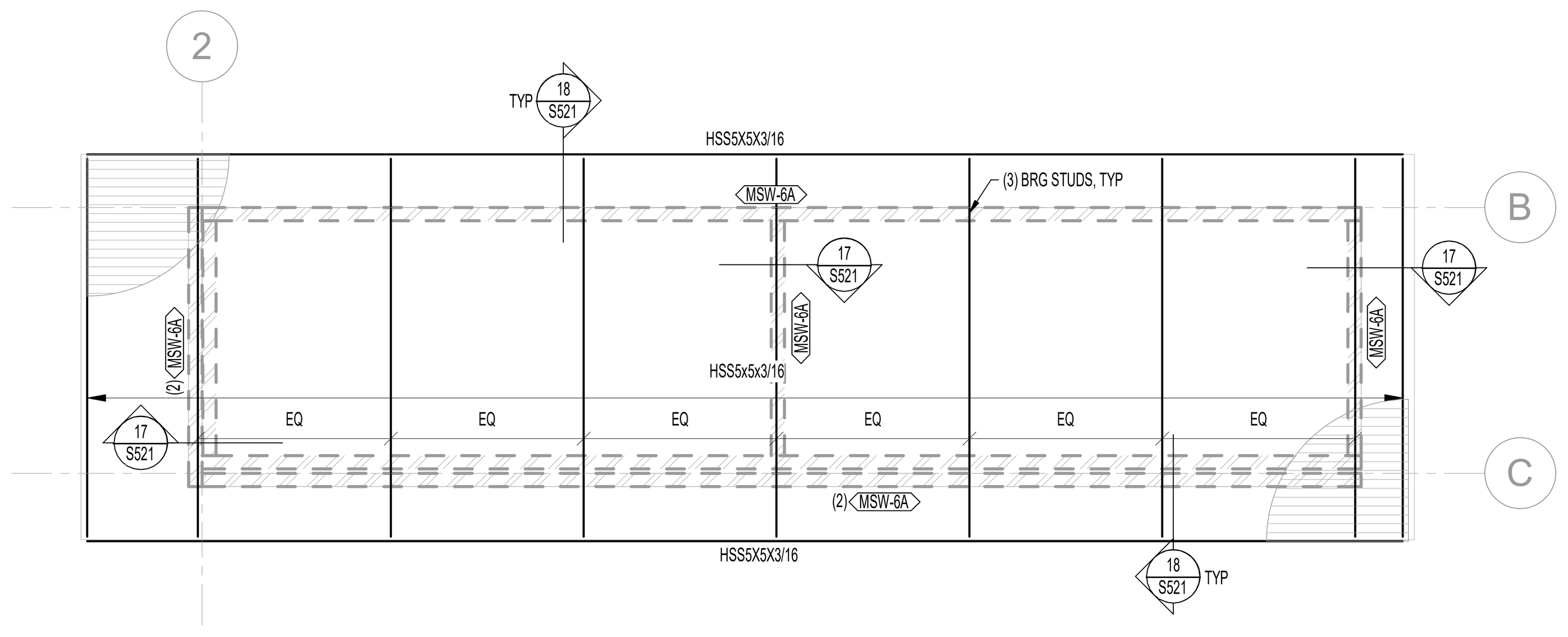
ROOF FRAMING PLAN NOTES

1. VERIFY ALL ROOF OPENINGS FOR MECHANICAL SHAFTS, DRAINS, ETC. WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. JOIST SUPPLIER SHALL DESIGN ALL ROOF JOIST BEARING ENDS AT MASONRY WALLS TO TRANSFER 8,000# (ALLOWABLE) AXIAL LOAD FOR 10" CMU WALLS AND 4,000# (ALLOWABLE) AXIAL LOAD FOR 8" CMU WALLS THROUGH JOIST BEARING SHOE.
3. ALL JOISTS SHALL HAVE 5" DEEP BEARING ENDS (UNO).
4. ALL ROOF OPENINGS GREATER THAN, OR EQUAL TO, 12" x 12" SHALL BE FRAMED AS INDICATED IN DETAILS 1/S521 AND 2/S521. FOR OPENINGS WHICH CUT LESS THAN TWO DECK FLUTES, SEE DETAIL 3/S521.
5. SEE DETAIL 4/S521 WHEN CONCENTRATED LOADS ARE LOCATED MORE THAN 6" FROM JOIST OR JOIST GIRDER PANEL POINT.
6. SEE DETAIL 5/S521 WHEN MECHANICAL UNITS ARE HUNG BELOW JOISTS.
7. VERIFY SIZE, WEIGHT, AND LOCATION OF ALL ROOF TOP MECHANICAL UNITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. SEE DETAIL 6/S521 FOR STEEL FRAMES AT ALL ROOF TOP EQUIPMENT. COORDINATE OPENINGS WITH MECHANICAL, ELECTRICAL, AND GENERAL CONTRACTORS.
8. LOCATE MISCELLANEOUS MECHANICAL OPENINGS BETWEEN JOISTS, NOT UNDERNEATH THEM.
9. OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION TO THE UNIFORM AND POINT LOADS SHOWN.
10. JOIST BRIDGING SHOWN ON PLANS IS FOR REPRESENTATION ONLY; ACTUAL SIZE, QUANTITY, AND LOCATION WILL BE DETERMINED BY THE JOIST SUPPLIER PER 'SJI' REQUIREMENTS. ALL BRIDGING AND BRIDGING ANCHORS NEED TO BE IN PLACE BEFORE APPLYING ANY LOADS. WHERE SKYLIGHT OR MECHANICAL UNITS/DUCTS INTERRUPT HORIZONTAL BRIDGING, PROVIDE CROSS BRIDGING AT JOIST SPACES ON EACH SIDE OF THE OPENING. WHERE DIAGONAL BRIDGING CONFLICTS WITH MECHANICAL DUCTS, REMOVE DIAGONAL BRIDGING AND REPLACE WITH HORIZONTAL BRIDGING AFTER ROOF DECK IS IN PLACE.
11. JOIST DESIGNER SHALL DESIGN JOISTS AND GIRDERS AND SUPPLY ADDITIONAL BRIDGING AS REQUIRED FOR UPLIFT DUE TO WIND. ASSUME:
    - 0.6DL = 12psf
    - 0.6WL = 25psf (UPLIFT)
    - 13psf NET UPLIFT (ASD)
    - NO 1/3 STRESS INCREASE ALLOWED.
12. SEE DETAIL 7/S501 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
13. SEE DETAIL 8/S501 FOR CONDITION AT RECESSES IN MASONRY WALLS.
14. SEE DETAIL 9/S501 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
15. SEE DETAIL 10/S501 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
16. ~~SEE ARCHITECTURAL PLANS FOR DIMENSIONS TO ALL STEEL COLUMNS.~~
17. PROVIDE HSS5x5x3/16xCONT BLOCKING OVER BEAM. BRACE BOTTOM FLANGE OF BEAM PER DETAIL 7/S511 AT 6'-0" O.C. MAX. BEAM RUNS CONTINUOUS OVER MASONRY WALL.



1 ROOF FRAMING PLAN - AREA B

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"



2 ROOF FRAMING PLAN - HIGH ROOF

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

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DATE: APRIL 15, 2024

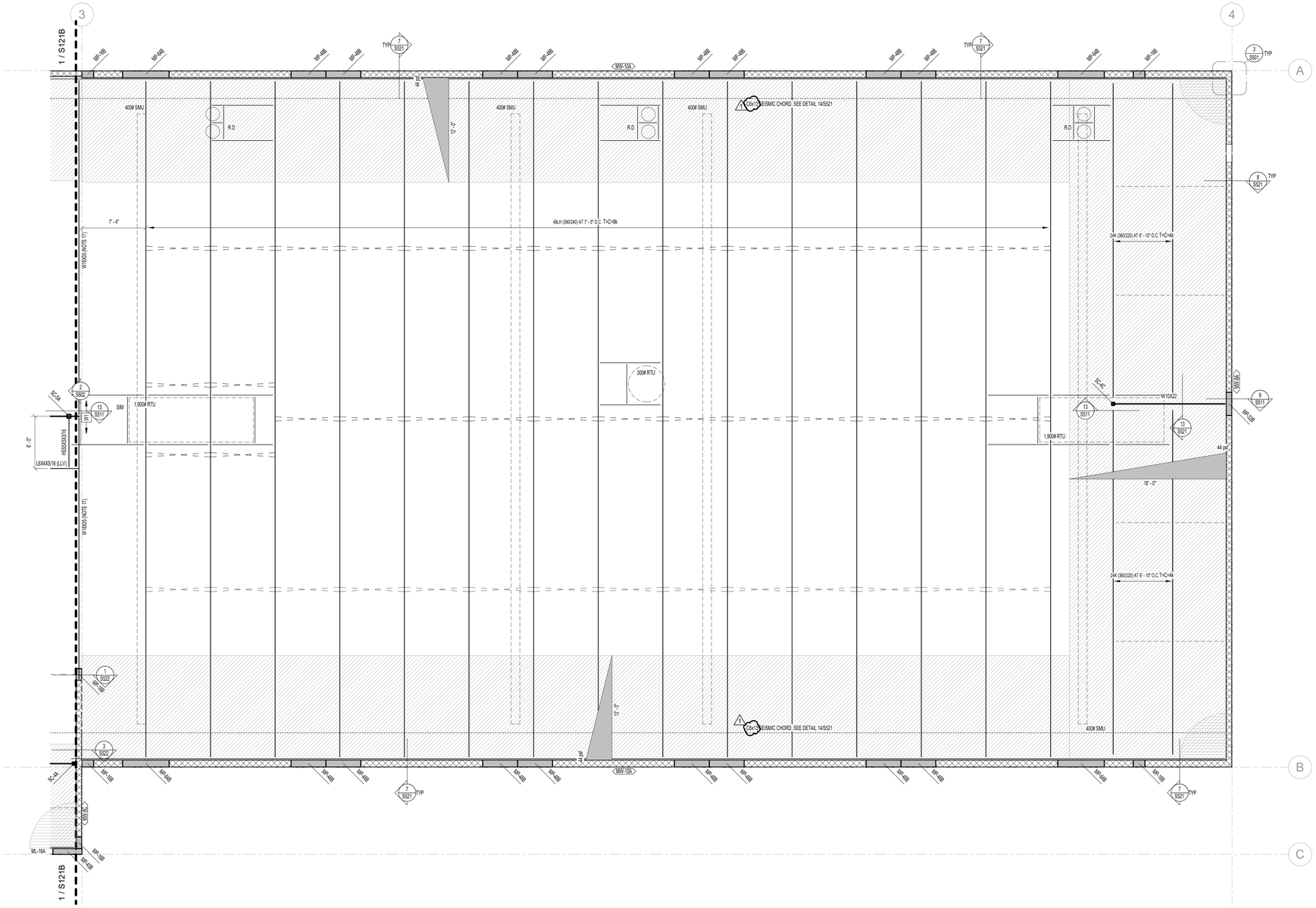
REVISIONS:

1 04/15/24 ADDENDUM 1

BID SET - SECTION 1

SHEET TITLE:
ROOF FRAMING PLAN - AREA B

SHEET NUMBER:
S121B



1 ROOF FRAMING PLAN - AREA C

3/16" = 1'-0" 0' 4'-0" 8'-0" 16'-0"

ROOF FRAMING DESIGN LOADS

ROOF LOADS	
DEAD LOAD	25psf
SNOW LOAD	32psf + DRIFT
TOTAL LOAD	57psf

1. AT OVERHANGS, INCREASE SNOW LOADING BY A FACTOR OF 2.4.

ROOF FRAMING PLAN NOTES

1. VERIFY ALL ROOF OPENINGS FOR MECHANICAL SHAFTS, DRAINS, ETC. WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. JOIST SUPPLIER SHALL DESIGN ALL ROOF JOIST BEARING ENDS AT MASONRY WALLS TO TRANSFER 8,000# (ALLOWABLE) AXIAL LOAD FOR 10" CMU WALLS AND 4,000# (ALLOWABLE) AXIAL LOAD FOR 8" CMU WALLS THROUGH JOIST BEARING SHOE.
3. ALL JOISTS SHALL HAVE 5" DEEP BEARING ENDS (UNO).
4. ALL ROOF OPENINGS GREATER THAN, OR EQUAL TO, 12" x 12" SHALL BE FRAMED AS INDICATED IN DETAILS 1/SS21 AND 2/SS21. FOR OPENINGS WHICH CUT LESS THAN TWO DECK FLUTES, SEE DETAIL 3/SS21.
5. SEE DETAIL 4/SS21 WHEN CONCENTRATED LOADS ARE LOCATED MORE THAN 6" FROM JOIST OR JOIST GIRDER PANEL POINT.
6. SEE DETAIL 5/SS21 WHEN MECHANICAL UNITS ARE HUNG BELOW JOISTS.
7. VERIFY SIZE, WEIGHT, AND LOCATION OF ALL ROOF TOP MECHANICAL UNITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. SEE DETAIL 6/SS21 FOR STEEL FRAMES AT ALL ROOF TOP EQUIPMENT. COORDINATE OPENINGS WITH MECHANICAL, ELECTRICAL, AND GENERAL CONTRACTORS
8. LOCATE MISCELLANEOUS MECHANICAL OPENINGS BETWEEN JOISTS, NOT UNDERNEATH THEM.
9. OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION TO THE UNIFORM AND POINT LOADS SHOWN.
10. JOIST BRIDGING SHOWN ON PLANS IS FOR REPRESENTATION ONLY; ACTUAL SIZE, QUANTITY, AND LOCATION WILL BE DETERMINED BY THE JOIST SUPPLIER PER 'SJI' REQUIREMENTS. ALL BRIDGING AND BRIDGING ANCHORS NEED TO BE IN PLACE BEFORE APPLYING ANY LOADS. WHERE SKYLIGHT OR MECHANICAL UNITS/DUCTS INTERRUPT HORIZONTAL BRIDGING, PROVIDE CROSS BRIDGING AT JOIST SPACES ON EACH SIDE OF THE OPENING. WHERE DIAGONAL BRIDGING CONFLICTS WITH MECHANICAL DUCTS, REMOVE DIAGONAL BRIDGING AND REPLACE WITH HORIZONTAL BRIDGING AFTER ROOF DECK IS IN PLACE.
11. JOIST DESIGNER SHALL DESIGN JOISTS AND GIRDERS AND SUPPLY ADDITIONAL BRIDGING AS REQUIRED FOR UPLIFT DUE TO WIND. ASSUME:
    - 0.6DL + 12psf
    - 0.6WL + 25psf (UPLIFT)
    - 15psf NET UPLIFT (ASD)
    - NO 1/3 STRESS INCREASE ALLOWED
12. SEE DETAIL 7/SS01 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
13. SEE DETAIL 8/SS01 FOR CONDITION AT RECESSES IN MASONRY WALLS.
14. SEE DETAIL 9/SS01 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
15. SEE DETAIL 10/SS01 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
16. SEE ARCHITECTURAL PLANS FOR DIMENSIONS TO ALL STEEL COLUMNS.
17. PROVIDE HSS5x5x3/16xCONT BLOCKING OVER BEAM. BRACE BOTTOM FLANGE OF BEAM PER DETAIL 7/SS11 AT 6'-0" O.C. MAX. BEAM RUNS CONTINUOUS OVER MASONRY WALL.

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT:
OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BHB PROJECT NO. 230074
DATE: APRIL 15, 2024

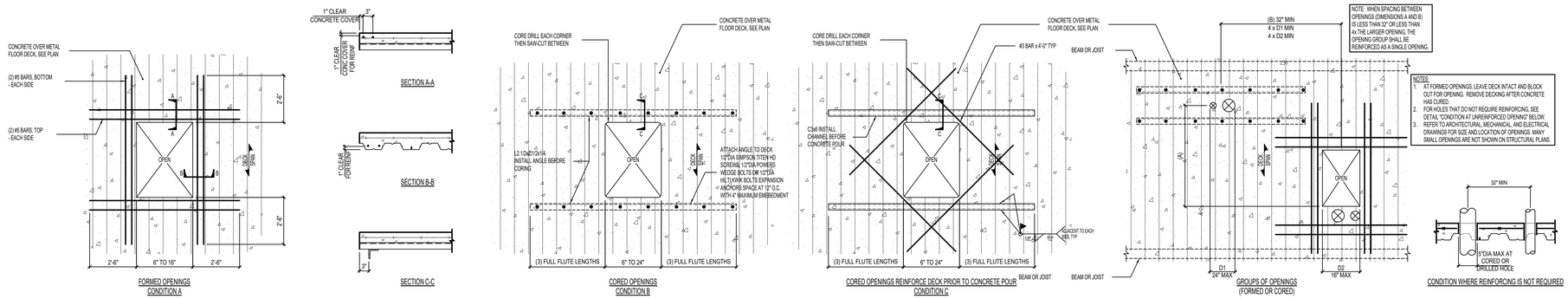
REVISIONS:

1 04/15/24 ADDENDUM 1

BID SET - SECTION 1

SHEET TITLE:
ROOF FRAMING PLAN - AREA C

SHEET NUMBER:
S121C



FORMED OPENINGS CONDITION A

CORED OPENINGS CONDITION B

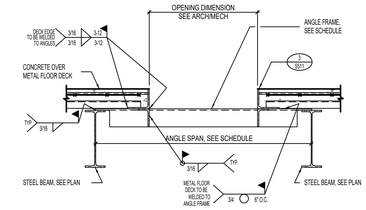
CORED OPENINGS REINFORCE DECK PRIOR TO CONCRETE POUR CONDITION C

GROUPS OF OPENINGS (FORMED OR CORED)

CONDITION WHERE REINFORCING IS NOT REQUIRED

1 MISCELLANEOUS SMALL OPENINGS IN FLOOR SLAB (UP TO 24")

NO SCALE

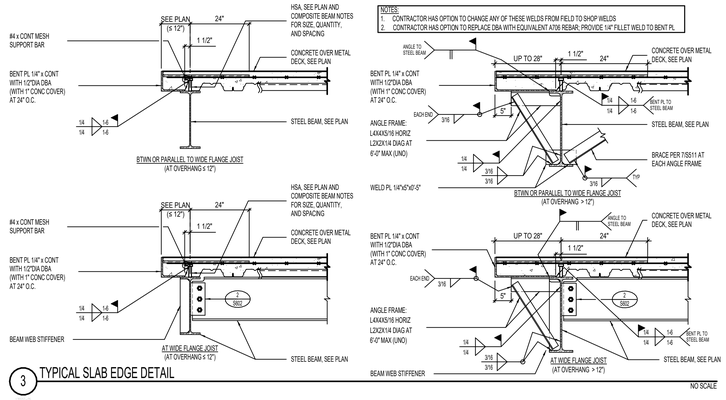


ANGLE FRAME SCHEDULE	
ANGLE (LLV)	MAXIMUM ANGLE SPAN (FT)
L5x5x1/4	9'-0"
L5x5x5/16	10'-0"
L5x3x3/8	11'-0"
L6x4x5/16	12'-0"

NOTE: FOR OPENINGS GREATER THAN 48", CONTACT STRUCTURAL ENGINEER

2 TYPICAL FLOOR OPENING DETAIL [OPENING < 48" AND > 24"]

NO SCALE



3 TYPICAL SLAB EDGE DETAIL

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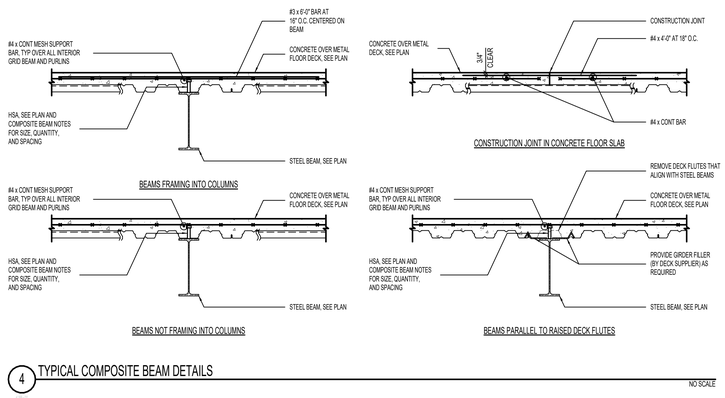
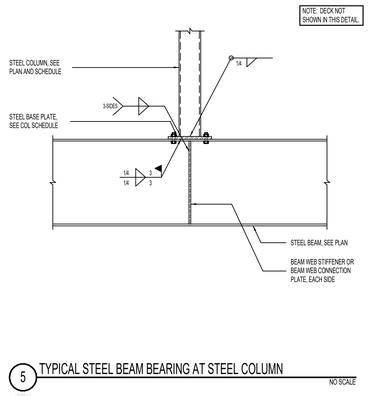


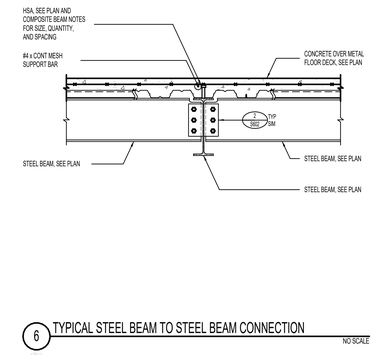
4 TYPICAL COMPOSITE BEAM DETAILS

NO SCALE



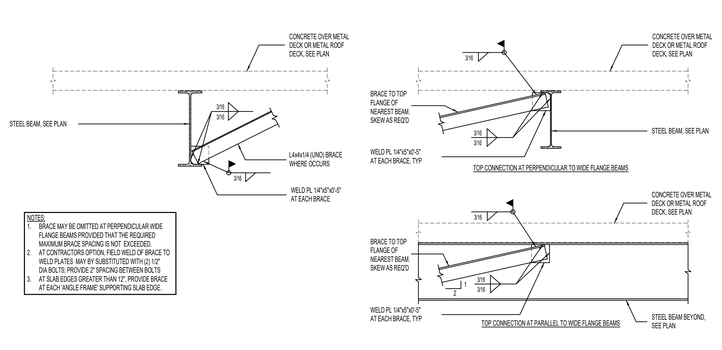
5 TYPICAL STEEL BEAM BEARING AT STEEL COLUMN

NO SCALE



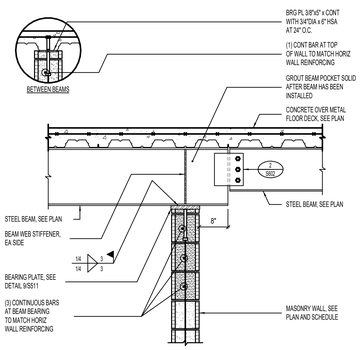
6 TYPICAL STEEL BEAM TO STEEL BEAM CONNECTION

NO SCALE



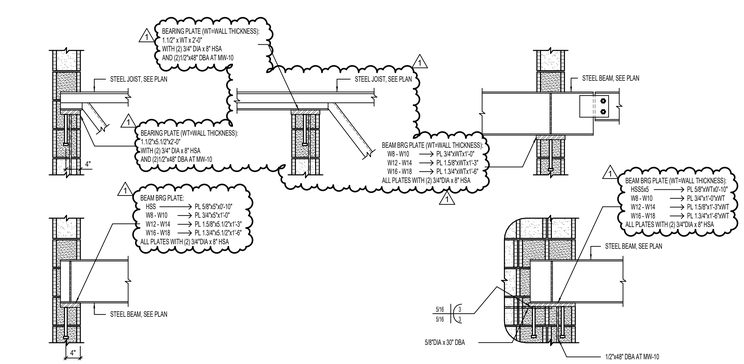
7 TYPICAL BRACE DETAIL

NO SCALE



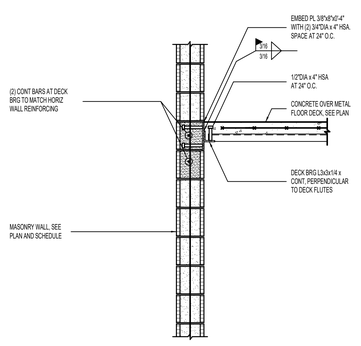
8 TYPICAL DROP-IN BEAM BEARING AT MASONRY WALL

NO SCALE



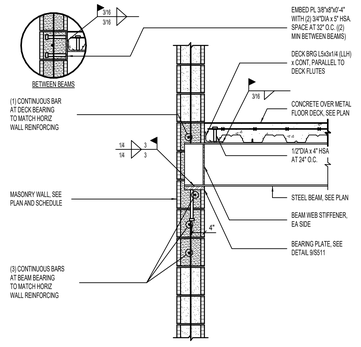
9 BEARING PLATE SCHEDULE FOR FLOOR JOIST/BEAM BEARING AT MASONRY WALLS

NO SCALE



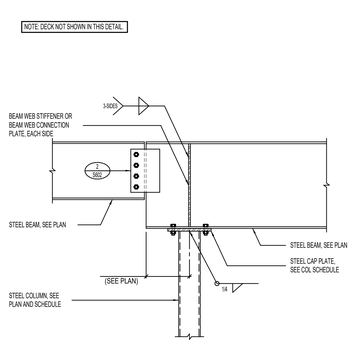
10 TYPICAL DECK BEARING AT MASONRY WALL

NO SCALE



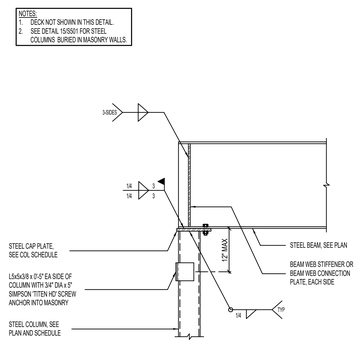
11 TYPICAL STEEL BEAM/DECK BEARING AT MASONRY WALL

NO SCALE



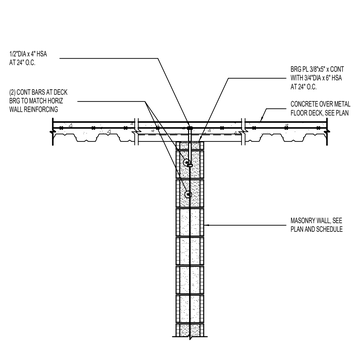
12 TYPICAL DROP-IN STEEL BEAM CONNECTION AT STEEL COLUMN

NO SCALE



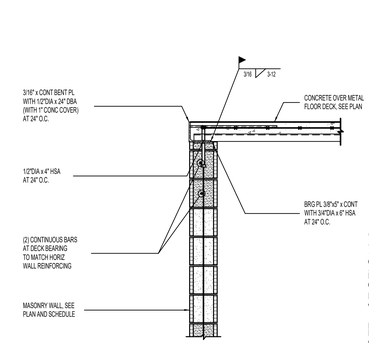
13 TYPICAL STEEL BEAM BEARING AT STEEL COLUMN

NO SCALE



14 TYPICAL DECK BEARING AT MASONRY WALL

NO SCALE



15 TYPICAL DECK BEARING AT MASONRY WALL

NO SCALE

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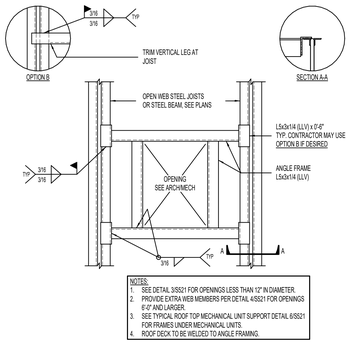
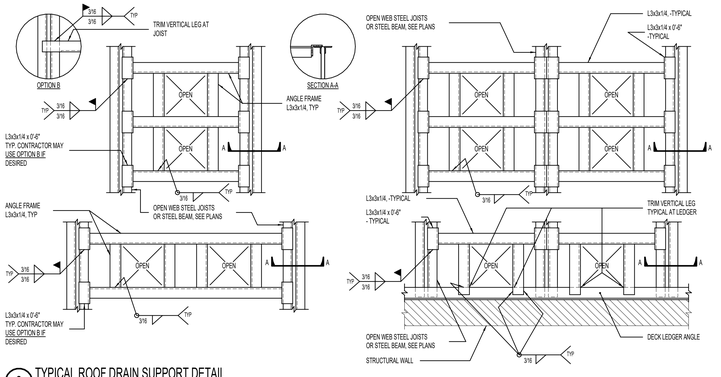
SHEET TITLE:
DETAILS

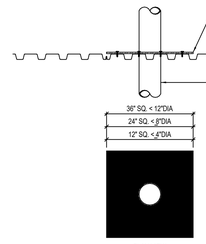
SHEET NUMBER:
S511



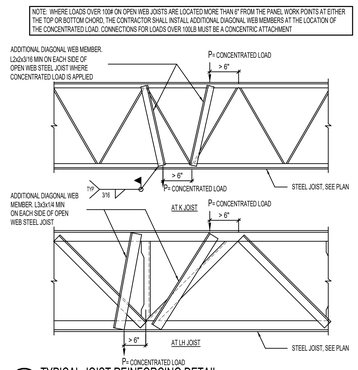
1 TYPICAL ROOF OPENING DETAIL [PLAN VIEW] — NO SCALE



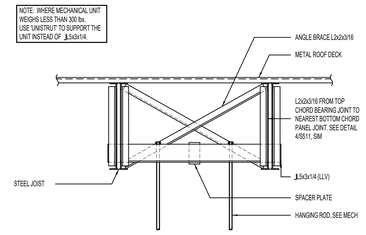
2 TYPICAL ROOF DRAIN SUPPORT DETAIL [PLAN VIEW] — NO SCALE



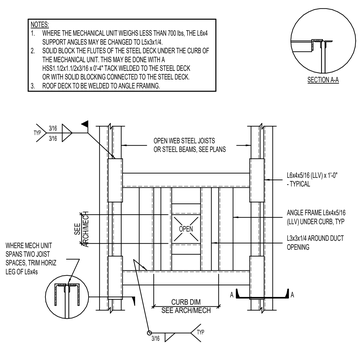
3 TYPICAL PIPE SLEEVE THROUGH ROOF DECK — NO SCALE



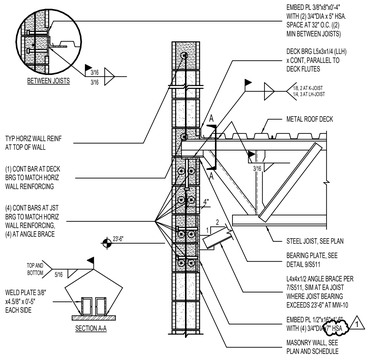
4 TYPICAL JOIST REINFORCING DETAIL FOR LOADS GREATER THAN 100 POUNDS — NO SCALE



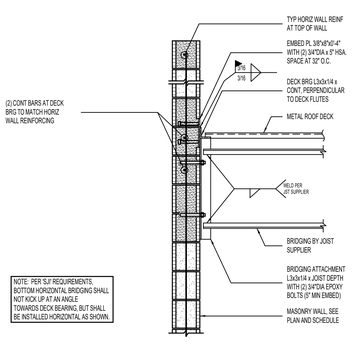
5 TYPICAL HANGING MECHANICAL UNIT SUPPORT DETAIL — NO SCALE



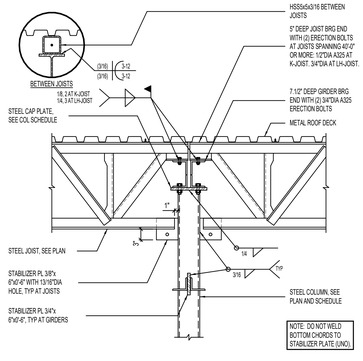
6 TYPICAL ROOF TOP MECHANICAL UNIT SUPPORT DETAIL [PLAN VIEW] — NO SCALE



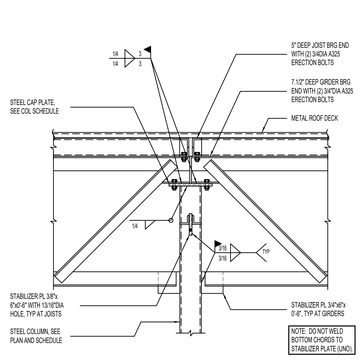
7 TYPICAL JOIST BEARING AT MASONRY WALL — NO SCALE



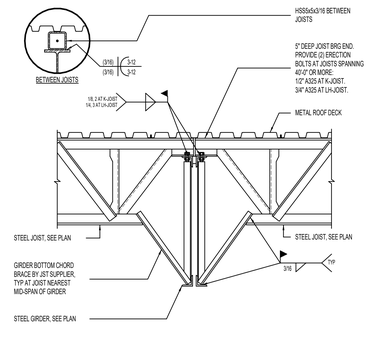
8 TYPICAL DECK BEARING WITH BRIDGING ATTACHMENT AT 8" MASONRY WALLS — NO SCALE



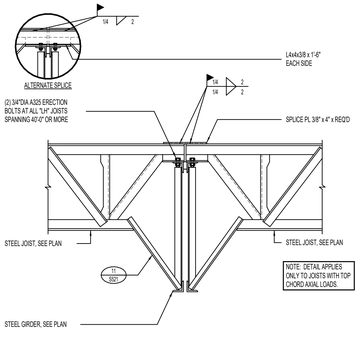
9 TYPICAL JOIST AND GIRDER BEARING AT STEEL COLUMN — NO SCALE



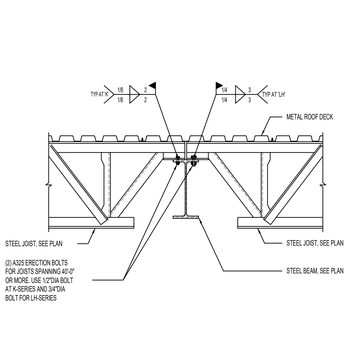
10 TYPICAL JOIST AND GIRDER BEARING AT STEEL COLUMN — NO SCALE



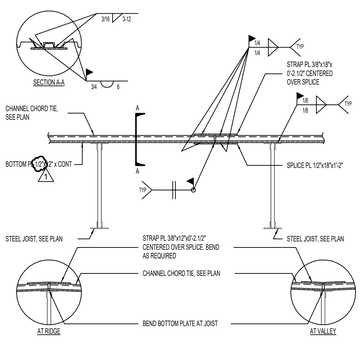
11 TYPICAL JOIST BEARING AT STEEL GIRDER — NO SCALE



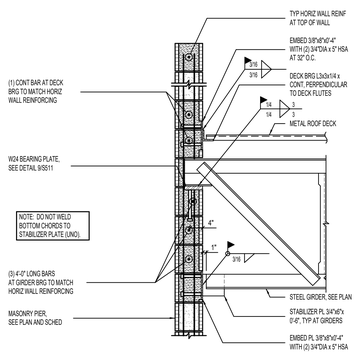
12 TYPICAL JOIST CHORD TIE/DRAG STRUT DETAIL — NO SCALE



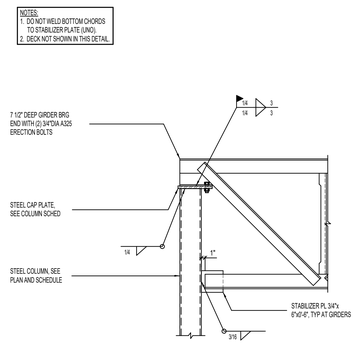
13 TYPICAL JOIST BEARING AT STEEL BEAM — NO SCALE



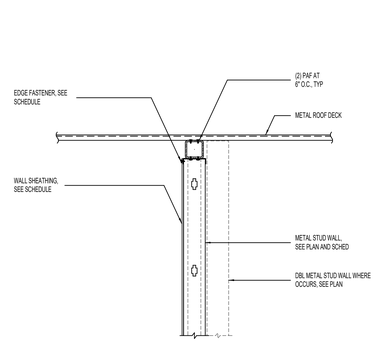
14 TYPICAL CHANNEL CHORD TIE DETAIL — NO SCALE



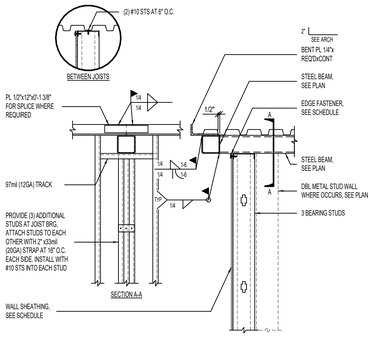
15 GIRDER BEARING DETAIL AT MASONRY PIER — NO SCALE



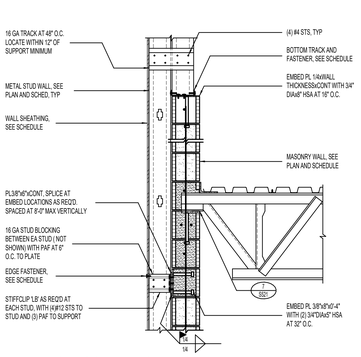
16 TYPICAL GIRDER BEARING AT STEEL COLUMN — NO SCALE



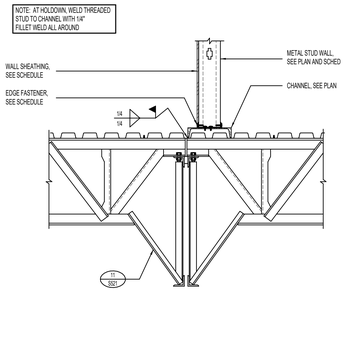
17 ROOF DECK BEARING ON METAL STUD SHEARWALL — NO SCALE



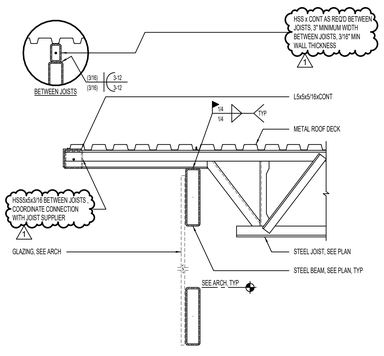
18 STEEL BEAM AT STEEL JOIST — NO SCALE



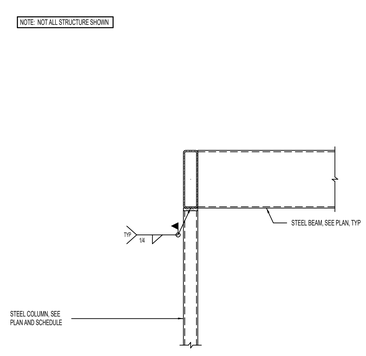
19 TYPICAL JOIST BEARING AT MASONRY WALL — NO SCALE



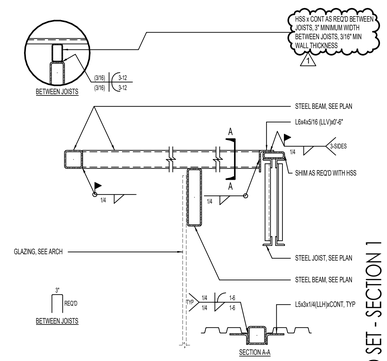
20 METAL STUD WALL AT ROOF STRUCTURE — NO SCALE



21 STEEL JOIST AT STEEL BEAM — NO SCALE



22 STEEL JOIST AT STEEL BEAM — NO SCALE



23 STEEL BEAM AT STEEL JOIST — NO SCALE

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: OPERATIONS BUILDING - SECTION 1

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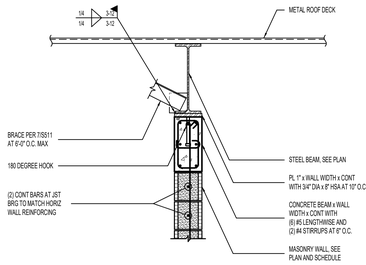
BID SET - SECTION 1

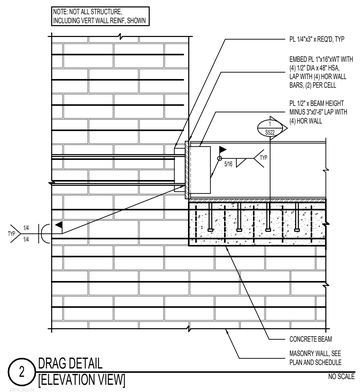
BHB PROJECT NO. 230074
DATE: APRIL 15, 2024
REVISIONS:
1 04/15/24 ADDENDUM 1

SHEET TITLE:
DETAILS

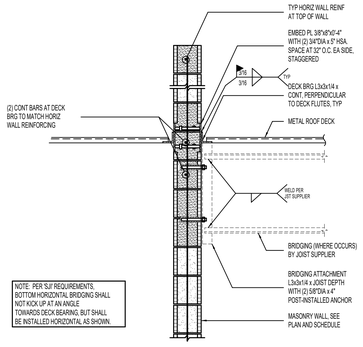
SHEET NUMBER:
S521



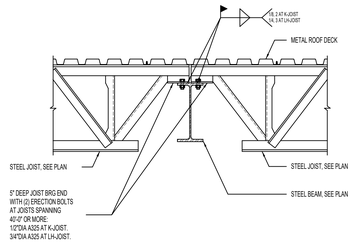
1 STEEL STRUCTURE AT MASONRY WALL — NO SCALE



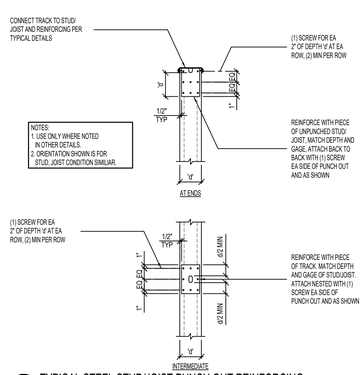
2 DRAG DETAIL [ELEVATION VIEW] — NO SCALE



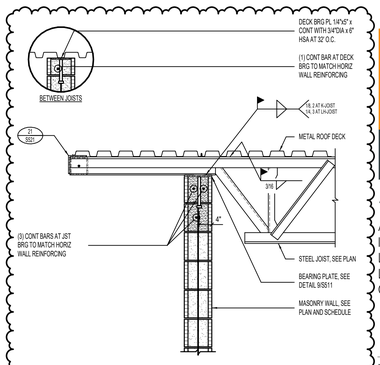
3 DECK BEARING WITH BRIDGING ATTACHMENT AT MASONRY WALL — NO SCALE



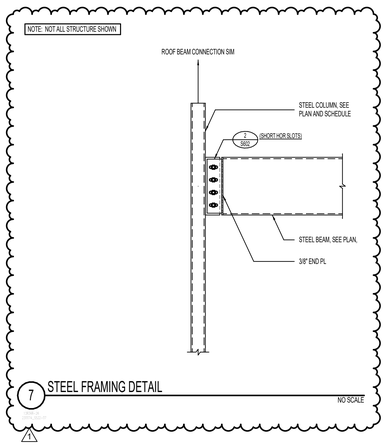
4 TYPICAL JOIST BEARING AT STEEL BEAM — NO SCALE



5 TYPICAL STEEL STUD/JOIST PUNCH OUT REINFORCING — NO SCALE



6 TYPICAL JOIST BEARING AT MASONRY WALL — NO SCALE



7 STEEL FRAMING DETAIL — NO SCALE

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BID SET - SECTION 1

BHB PROJECT NO. 230074
DATE: APRIL 15, 2024

REVISIONS:
1 04/15/24 ADDENDUM 1

SHEET TITLE:
DETAILS

SHEET NUMBER:
S522

CONCRETE THICKENED SLAB FOOTING SCHEDULE (FTS)

MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE No.	SIZE	LENGTH	SPACING	REINFORCING LENGTHWISE No.	SIZE	LENGTH	SPACING	COMMENTS
FTS2.0	2' - 0"	CONT	12"	-	#4	1' - 6"	48"	3	#4	CONT	EQ	

CONCRETE CONTINUOUS FOOTING SCHEDULE (FC)

MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE No.	SIZE	LENGTH	SPACING	REINFORCING LENGTHWISE No.	SIZE	LENGTH	SPACING	COMMENTS
FC2.0	2' - 0"	CONT	12"	-	#4	1' - 6"	48"	3	#4	CONT	EQ	
FC2.5	2' - 6"	CONT	12"	-	#5	2' - 0"	14"	3	#5	CONT	EQ	
FC3.0	3' - 0"	CONT	12"	-	#5	2' - 6"	14"	3	#5	CONT	EQ	
FC3.0A	3' - 0"	CONT	14"	-	#5	2' - 6"	12"	3	#5	CONT	EQ	REINFORCE TOP AND BOTTOM
FC4.0	4' - 0"	CONT	12"	-	#5	3' - 6"	14"	4	#5	CONT	EQ	REINFORCE TOP AND BOTTOM

CONCRETE SPOT FOOTING SCHEDULE (FS)

MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE No.	SIZE	LENGTH	SPACING	REINFORCING LENGTHWISE No.	SIZE	LENGTH	SPACING	COMMENTS
FS3.0	3' - 0"	3' - 0"	12"	3	#5	2' - 6"	EQ	3	#5	2' - 6"	EQ	
FS4.0	4' - 0"	4' - 0"	12"	4	#5	3' - 6"	EQ	4	#5	3' - 6"	EQ	
FS4.5	4' - 6"	4' - 6"	12"	4	#5	4' - 0"	EQ	4	#5	4' - 0"	EQ	
FS5.0	5' - 0"	5' - 0"	12"	5	#5	4' - 6"	EQ	5	#5	4' - 6"	EQ	
FS6.0	6' - 0"	6' - 0"	12"	6	#5	5' - 6"	EQ	6	#5	5' - 6"	EQ	
FS8.0	8' - 0"	8' - 0"	15"	6	#6	7' - 6"	EQ	6	#6	7' - 6"	EQ	
FS10.5	10' - 6"	10' - 6"	19"	11	#6	10' - 0"	EQ	11	#6	10' - 0"	EQ	

CONCRETE FOOTING NOTES:
1. PLACE ALL FOOTING REINFORCING IN THE BOTTOM OF THE FOOTING WITH 3" CLEAR CONCRETE COVER (UNO).
2. TOP REINFORCING, WHERE OCCURS, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
3. IF FOOTINGS ARE EARTH-FORMED, FOOTINGS SHALL BE 6" LONGER AND WIDER THAN SCHEDULED.
4. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
5. SOME SCHEDULED FOOTINGS MAY NOT BE USED, SEE FOOTING AND FOUNDATION PLAN FOR FOOTING MARKS.

1 CONCRETE FOOTING SCHEDULE (C4000-S3000)

CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE

BAR SIZE	f'c = 3000psi & f'c = 3500 psi				f'c = 4000psi & f'c = 4500 psi				f'c = 5000psi				f'c = 6000psi			
	REGULAR CLASS		TOP CLASS		REGULAR CLASS		TOP CLASS		REGULAR CLASS		TOP CLASS		REGULAR CLASS		TOP CLASS	
	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B
#3	17"	22"	22"	28"	15"	19"	19"	24"	13"	17"	17"	22"	12"	16"	15"	20"
#4	22"	29"	29"	37"	19"	25"	25"	32"	17"	22"	22"	29"	16"	20"	20"	27"
#5	28"	36"	36"	47"	24"	31"	31"	40"	22"	28"	28"	36"	20"	26"	26"	33"
#6	33"	43"	43"	56"	29"	37"	37"	48"	26"	33"	33"	43"	24"	31"	31"	40"
#7	48"	63"	63"	81"	42"	54"	54"	70"	37"	49"	49"	63"	34"	44"	44"	58"
#8	55"	72"	72"	93"	48"	62"	62"	80"	43"	56"	55"	72"	39"	51"	51"	66"
#9	62"	81"	81"	105"	54"	70"	70"	91"	48"	63"	63"	81"	44"	57"	57"	74"
#10	70"	91"	91"	118"	61"	79"	79"	102"	54"	70"	70"	91"	50"	64"	64"	83"
#11	78"	101"	101"	131"	67"	87"	87"	113"	60"	78"	78"	101"	55"	71"	71"	93"

TABULATED VALUES ARE FOR CASE 1 REINFORCEMENT, WHERE THE REQUIREMENTS OF TABLE BELOW ARE MET. WHERE THESE CONDITIONS ARE NOT MET, MULTIPLY THE LAP LENGTHS (^{f}d) BY 1.5.

REQUIREMENT FOR CASE 1 LAP LENGTHS

BAR CLEAR SPACING	CLEAR COVER	STIRRUPS OR TIES
>=db	>=db	>=CODE FOR MINIMUM THROUGHOUT ^{f}d
>=2db	>=db	NO REQUIREMENT

db = BAR DIAMETER

CONCRETE REINFORCING BAR LAP SPLICE NOTES:
1. THIS SCHEDULE SHALL BE USED FOR ALL BAR SPLICES IN CONCRETE WALLS, UNLESS NOTED OTHERWISE.
2. CLASS 'A' SPLICES MAY BE USED ONLY IN CASES WHERE 50% OR LESS OF THE BARS ARE SPLICED WITHIN THE LAP SPLICE LENGTH.
3. CLASS 'B' SPLICES SHALL BE USED FOR ALL SPLICES UNLESS THE REQUIREMENTS OF NOTE No. 2 ABOVE ARE MET.
4. TIES AND STIRRUPS SHALL NOT BE SPLICED.
5. DO NOT SPLICE VERTICAL BARS IN RETAINING WALLS UNLESS SPECIFICALLY SHOWN.
6. THE VALUES TABULATED IN SCHEDULE ARE FOR GRADE 60 REINFORCING BARS. FOR GRADE 75, MULTIPLY LAP LENGTHS BY 1.25 AND FOR GRADE 80, MULTIPLY BY 1.33.
7. THE VALUES TABULATED IN SCHEDULE ARE MINIMUM REQUIREMENTS. LONGER LENGTHS MAY BE USED FOR CONSTRUCTIBILITY.
8. LAP SPLICES ARE NOT ALLOWED FOR BARS GREATER THAN #11 BAR. THE LENGTHS IN SCHEDULE ARE FOR TENSION DEVELOPMENT LENGTH.
9. TOP BARS ARE CLASSIFIED AS HORIZONTAL BARS WHERE 12", OR MORE, OF FRESH CONCRETE IS CAST BELOW THE REINFORCING BAR.
10. FOR EPOXY-COATED OR ZINC AND EPOXY DUAL-COATED BARS WITH CLEAR COVER < 3db OR CLEAR SPACING <6db , MULTIPLY LAP LENGTHS BY 1.5. FOR ALL OTHER CASES MULTIPLY BY 1.2
11. FOR LIGHT WEIGHT CONCRETE, MULTIPLY LAP LENGTHS BY 1.33 UNLESS THE AVERAGE SPLITTING TENSILE STRENGTH (Fct) IS SPECIFIED. FOR LIGHT WEIGHT CONCRETE WHERE Fct IS SPECIFIED, REFER TO ACI318-14 SECTION 19.2.4.3
12. SPLICES FOR BUNDLED BARS:
    A. FOR BUNDLED BARS OF THREE OR LESS, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.2.
    B. FOR BUNDLED BARS OF FOUR OR MORE, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.33.
    C. INDIVIDUAL BAR SPLICES WITHIN A BUNDLE SHALL NOT OVERLAP.
    D. ENTIRE BUNDLES SHALL NOT BE LAP SPLICED.
13. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

2 CONCRETE REINFORCING 01

CONCRETE WALL SCHEDULES

MARK	THICKNESS	REINFORCING VERTICAL	HORIZONTAL	TOP AND BOTTOM	WALL TYPE	COMMENTS
CW-8A	8"	#4 AT 16" O.C.	#4 AT 12" O.C.	(1) #4	A	
CW-8B	8"	#4 AT 32" O.C. (NOTE 2)	#4 AT 12" O.C.	(1) #4	A	
CW-8C	8"	VERTICAL MASONRY DOWELS	#4 AT 12" O.C.	(1) #4	A	
CW-8D	8"	VERTICAL MASONRY DOWELS	#4 AT 12" O.C.	(1) #4	A	
CW-10B	10"	4# AT 24" O.C. (NOTE 2)	#5 AT 15" O.C.	(1) #5	A	
CW-14A	14"	#4 AT 32" O.C. I.F. (NOTE 2) #4 AT 16" O.C. O.F.	#4 AT 12" O.C. E.F.	(2) #4	A	

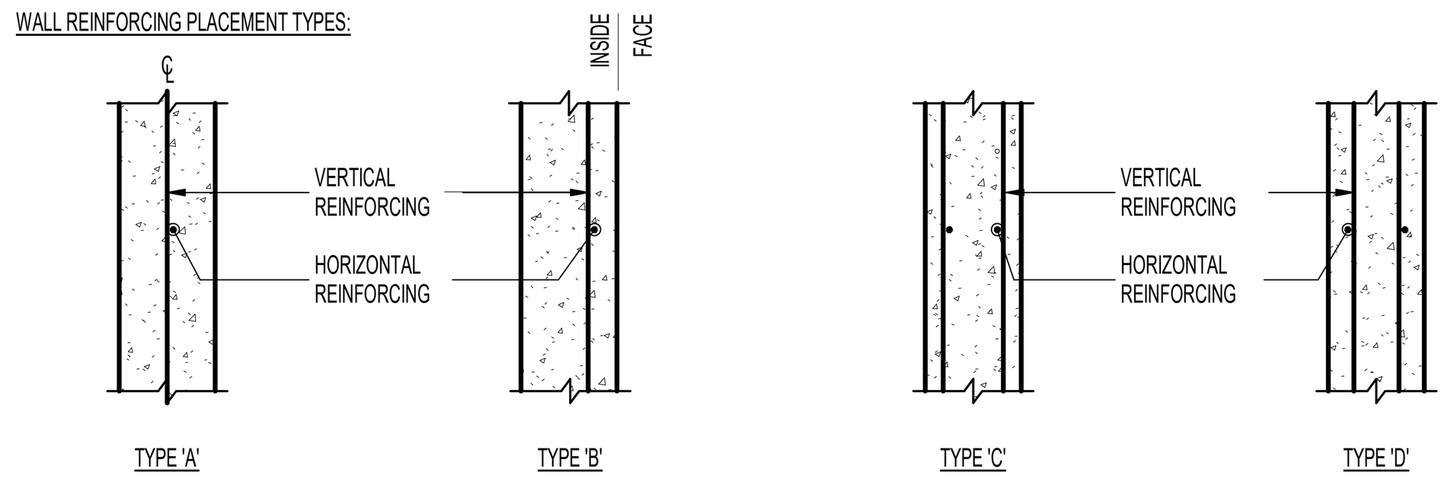
CONCRETE FOUNDATION WALL NOTES:
1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. STAGGER CONCRETE VERTICAL REINFORCING WITH MASONRY VERTICAL DOWELS. NET SPACING OF VERTICAL REINFORCING SHALL BE SPECIFIED SPACING DIVIDED BY HALF.

ABBREVIATIONS:
EACH FACE E.F.
INSIDE FACE I.F.
OUTSIDE FACE O.F.

WALLS NOT DESIGNATED IN PLAN

THICKNESS	REINFORCING VERTICAL	HORIZONTAL
6"	#4 BARS AT 18" O.C.	#4 BARS AT 16" O.C.
8"	#4 BARS AT 18" O.C.	#4 BARS AT 12" O.C.
10"	#4 BARS AT 16" O.C.	#5 BARS AT 15" O.C.
12"	#4 BARS AT 18" O.C. E.F.	#4 BARS AT 16" O.C. E.F.

WALL REINFORCING PLACEMENT TYPES:

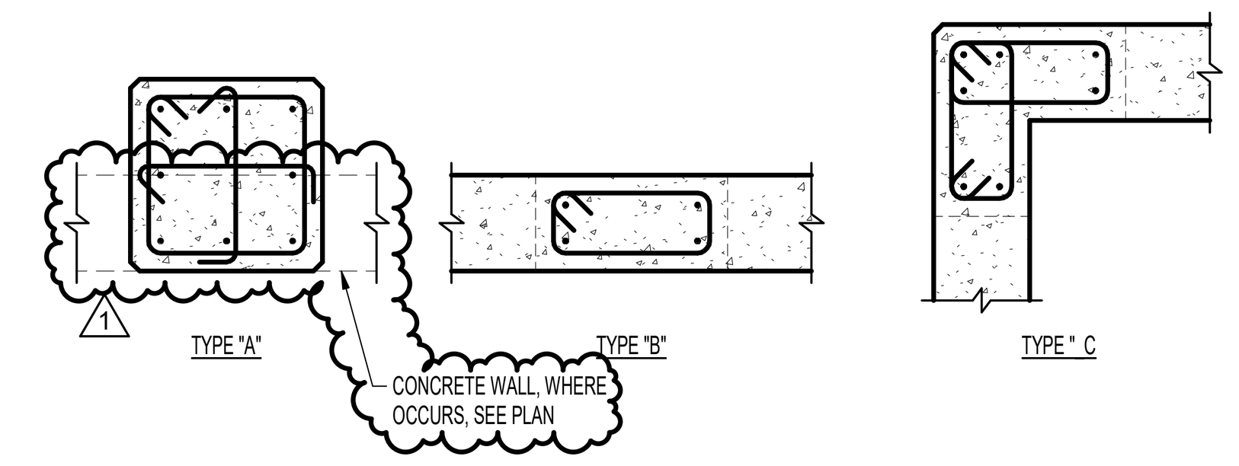


3 CONCRETE WALL SCHEDULE

CONCRETE PIER SCHEDULE

MARK	PIER SIZE W	x	L	REINFORCING VERTICAL	TIES	TYPE	COMMENTS
CP-16A	WT [illegible]	x	16"	(4) #5	(1) #3 AT 8" O.C.	B	
CP-16B	16"	x	16"	(7) #5	(2) #3 AT 8" O.C.	C	
CP-24A	24"	x	24"	(8) #6	(3) #3 AT 8" O.C.	A	

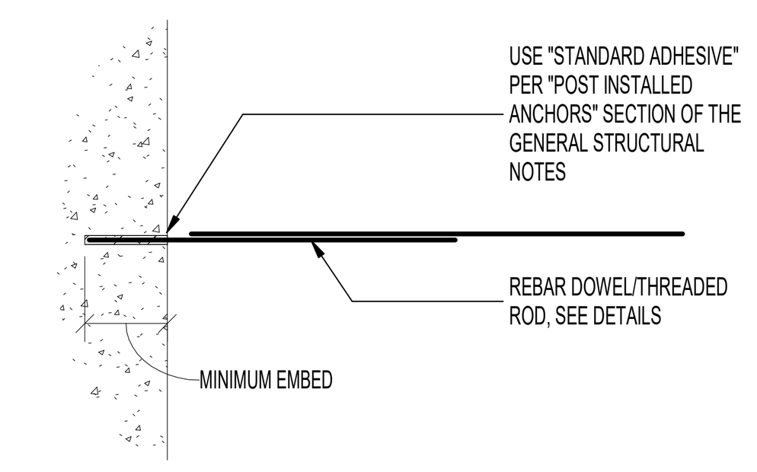
CONCRETE PIER NOTES:
1. INSTALL (3) SETS OF TIES WITHIN TOP 5" OF ALL PIERS (UNO).
2. RUN HORIZONTAL CONCRETE WALL REINFORCING CONTINUOUS THROUGH PIER WHEN PIER IS POURED MONOLITHICALLY WITH CONCRETE WALL.
3. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



4 CONCRETE PIER SCHEDULE

STANDARD ADHESIVE EMBEDMENT SCHEDULE

REBAR DOWEL (THREADED ROD SIZE)	MIN EMBEDMENT INTO CONCRETE OR GROUTED MASONRY
#3 (3/8")	3 3/8"
#4 (1/2")	4 1/2"
#5 (5/8")	5 5/8"
#6 (3/4")	6 3/4"



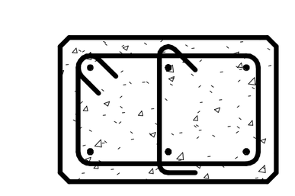
STANDARD ADHESIVE EMBEDMENT NOTES:
1. SPECIFIC EMBEDMENTS, NOTES AND DETAILS IN DRAWINGS SHALL GOVERN OVER THIS SCHEDULE.
2. HOLE DIAMETER SHALL BE DOWEL/ROD DIAMETER PLUS 1/8". FOLLOW MANUFACTURER'S INSTRUCTIONS FOR HOLE PREPARATION.
3. PROVIDE A 3" MINIMUM EDGE DISTANCE TO CENTER OF HOLE.
4. CONTACT STRUCTURAL ENGINEER IF MINIMUM EMBEDMENTS INDICATED ABOVE ARE NOT ACHIEVABLE.
5. SEE "POST INSTALLED ANCHORS" SECTION OF GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

5 STANDARD ADHESIVE EMBEDMENT SCHEDULE

CONCRETE GRADE BEAM SCHEDULE

3D	BEAM SIZE H	x	W	REINFORCING LENGTHWISE	TIES	TYPE	COMMENTS
GB-18A	18"		12"	(6) #6	(2) #3 TIES AT 6" O.C.	A	

CONCRETE GRADE BEAM NOTES:
1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



TYPE "A"

6 CONCRETE GRADE BEAM SCHEDULE

3/4" = 1'-0"

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT:
OPERATIONS BUILDING - SECTION 1
497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BHB PROJECT NO. 230074
DATE: APRIL 15, 2024
REVISIONS:
1 04/15/24 ADDENDUM 1

BID SET - SECTION 1

SHEET TITLE:
SCHEDULES

SHEET NUMBER:
S601

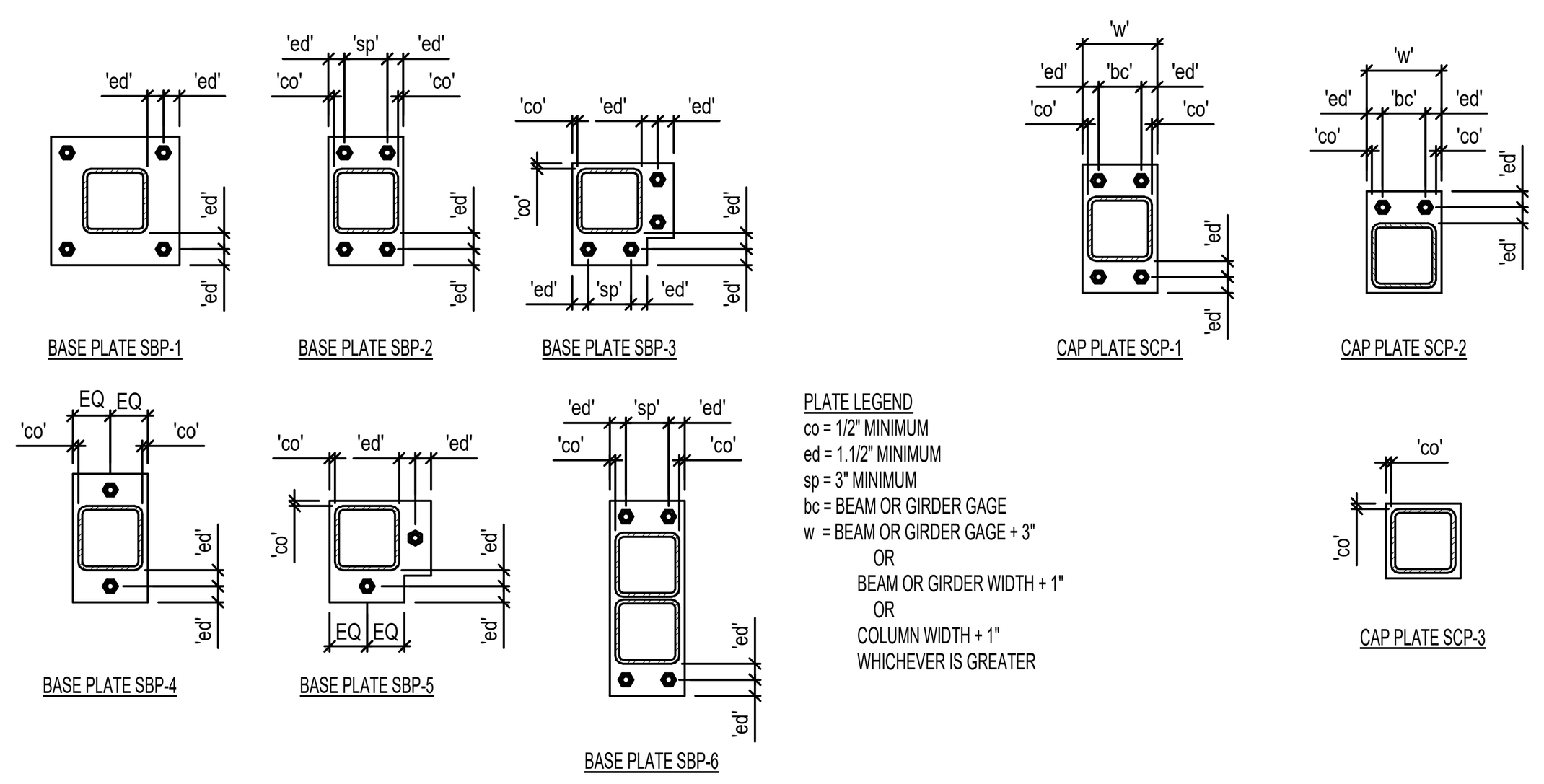
STEEL COLUMN SCHEDULE

MARK	SIZE	STEEL BASE PLATE	STEEL CAP PLATE	COMMENTS
SC-4A	HSS4x4x3/16	3/4" (SBP-2)	1/2" (SCP-2)	.
SC-4B	HSS4x4x3/16	3/4" (SBP-3)	1/2" (SCP-3)	
SC-4C	HSS4x4x1/4	3/4" (SBP-2)	1/2" (SCP-2)	
SC-4D	HSS4x4x1/2	3/4" (SBP-2)	1/2" (SCP-1)	
SC-4E	(2) HSS4x4x1/2	1" (SBP-6)	1/2" (SCP-2)	
SC-5A	HSS5x5x1/4	3/4" (SBP-2)	1/2" (SCP-2)	
SC-5B	HSS5x5x1/4	3/4" (SBP-2)	1/2" (SCP-3)	NOTE 6
SC-6A	HSS6x6x1/4	3/4" (SBP-2)	1/2" (SCP-1)	
SC-6B	HSS6x6x3/8	1" (SBP-2)	1/2" (SCP-2)	
SC-6C	(2) HSS6x6x1/2	1.1/2" (SBP-6)	1/2" (SCP-1)	

STEEL COLUMN NOTES:
1. UNLESS NOTED OTHERWISE, ALL COLUMNS SHALL BE INSTALLED WITH (4) 3/4"DIA ANCHOR RODS WITH 3" MINIMUM HOOKS. PROJECT ANCHOR RODS 3" MINIMUM ABOVE THE TOP OF THE BASE PLATE. EMBEDMENT SHALL BE 9" MINIMUM. ALL RODS SHALL BE INSTALLED WITH HARDENED WASHERS BENEATH THE NUT. ANY BOLT HOLES LARGER THAN THE ROD DIAMETER PLUS 5/16" SHALL HAVE 5/16" PLATE WASHERS INSTALLED BENEATH THE HARDENED WASHERS.
2. ALL CAP PLATE BOLTS SHALL BE 3/4"DIA A325N BOLTS, TYPICAL UNLESS NOTED OTHERWISE.
3. ANCHOR RODS SHALL NOT BE WELDED (INCLUDING TACK WELDS).
4. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
5. SEE DETAIL 15/S501 FOR COLUMNS EMBEDDED IN MASONRY WALLS.
6. USE EMBED PLATE WITH 3/4" DIA x 8" HSA.

STEEL BASE PLATE TYPES:

STEEL CAP PLATE TYPES:



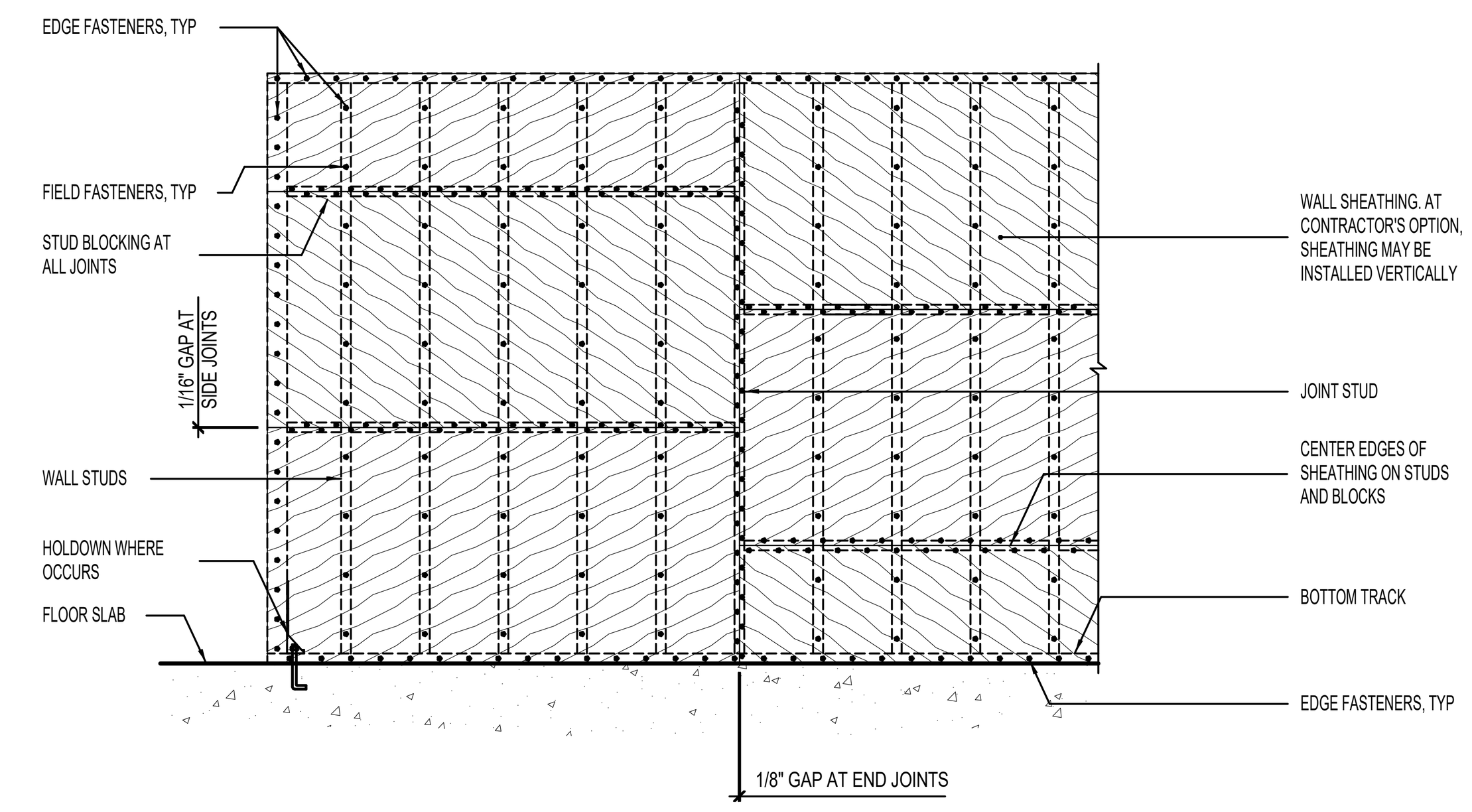
1 STEEL COLUMN SCHEDULE

NO SCALE

METAL STUD SHEARWALL WITH WOOD SHEATHING SCHEDULE

MARK	WALL FRAMING				WALL SHEATHING				COMMENTS
	METAL STUDS	TOP TRACK	BOTTOM TRACK	BOTTOM TRACK FASTENERS	THICKNESS	SCREW SIZE	EDGE FASTENER	FIELD FASTENER	
MSW-6A	600S162-43 AT 16" O.C.	600T125-54	600T125-54	5/8" x 4" AT 32" O.C.	7/16"	#8	6" O.C.	12" O.C.	

METAL STUD SHEARWALL NOTES:
1. BOTTOM TRACK FASTENERS TO BE CONCRETE SCREW ANCHOR OR CONCRETE WEDGE ANCHOR SUITABLE FOR CRACKED CONCRETE.
2. SCREWS SHALL HAVE A MINIMUM HEAD DIAMETER OF .292", IN ACCORDANCE WITH SAE J78.
3. SCREWS SHALL PENETRATE THROUGH FRAMING MEMBER WITH AT LEAST THREE THREADS.
4. STUDS SHALL BE A MINIMUM OF 1.5/8" WIDE WITH A 3/8" MINIMUM RETURN LIP.
5. TRACKS SHALL BE A MINIMUM OF 1.1/4" WIDE.
6. FOR STUD AND TRACK THICKNESSES GREATER THAN 43 MIL. USE Fy=50 KSI STEEL, OTHERWISE USE Fy=33 KSI STEEL.
7. HORIZONTAL BLOCKING SHALL BE PROVIDED AT 4'-0" O.C. MAX.
8. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
9. WEB PUNCH OUTS LESS THAN 1'-0" AWAY FROM EITHER END OF STUD SHALL BE REINFORCED PER DETAIL 5/S522.



3 METAL STUD SHEARWALL WITH WOOD SHEATHING SCHEDULE
[ELEVATION VIEW]

NO SCALE

METAL STUD HOLDOWN SCHEDULE

SIMPSON HOLDOWN	BOUNDARY MEMBER	BOUNDARY MEMBER FASTENERS	DETAIL	COMMENTS
S/HDU4	(2)600S162-54	(6)#14 SCREWS	20/S521	-

HOLDOWN NOTES:
1. ALL HOLDOWNS SPECIFIED ARE "SIMPSON - STRONG TIE", SEE GENERAL STRUCTURAL NOTES FOR SUBSTITUTIONS.
2. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

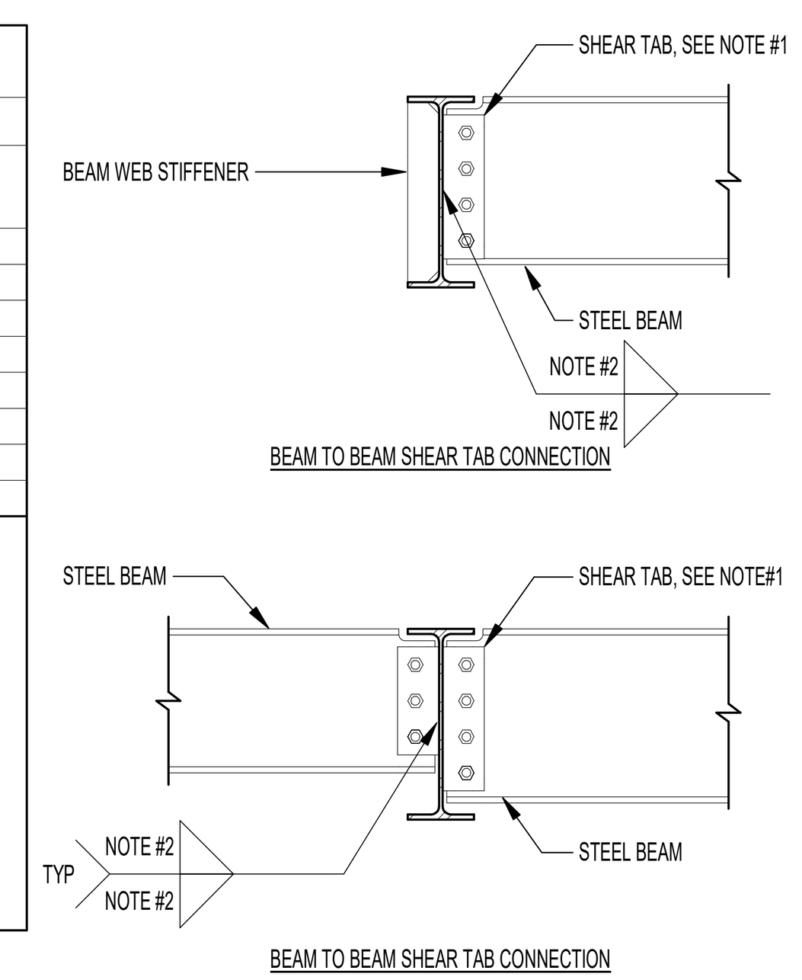
4 METAL STUD HOLDOWN SCHEDULE

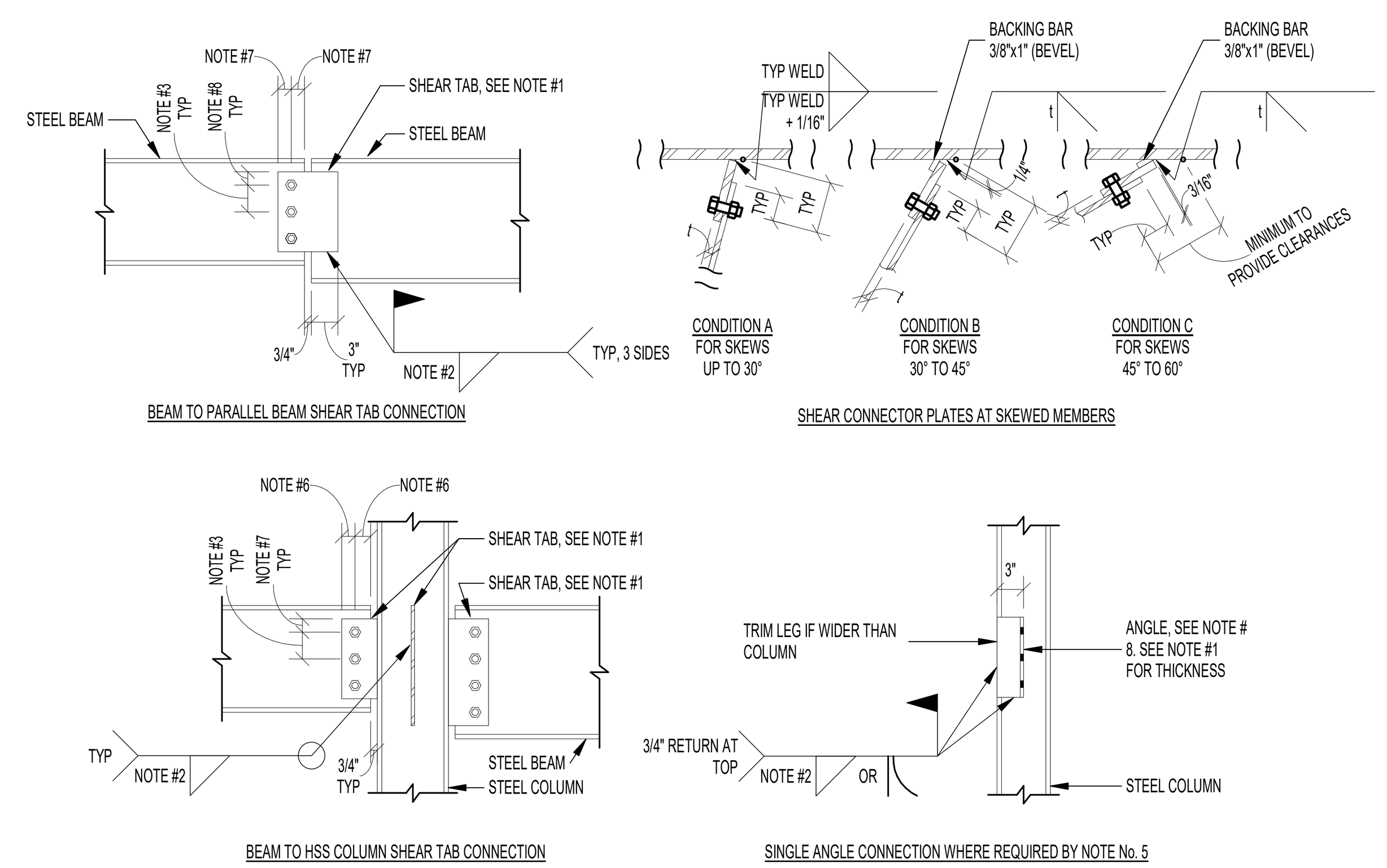
NO SCALE

A-490 BOLT SCHEDULE

Max Beam Size in Each Beam Depth Group	A-325N Bolts	
	No. Per Beam	Size
W8	2	7/8"DIA
W10	2	7/8"DIA
W12	3	7/8"DIA
W14	3	7/8"DIA
W16	4	7/8"DIA
W18	5	7/8"DIA
W21	6	7/8"DIA

1. SHEAR TAB (50ksi) SHALL BE 1/2" THICK.
2. 5/16" FILLET WELD EACH SIDE OF SHEAR TAB.
3. BOLT SPACING SHALL BE 3" TYP.
4. WHEN MORE THAN ONE ROW OF BOLTS IS NEEDED, THE FIRST ROW SHALL BE A COMPLETE ROW WITH THE REMAINDER OF THE BOLTS PLACED IN A SECOND ROW.
5. HSS COLUMN SHALL HAVE A MINIMUM 1/4" THICKNESS USE A SINGLE ANGLE CONNECTION WHERE STEEL TUBE WALL IS TOO THIN.
6. AT MOMENT FRAME COLUMNS, SEE MOMENT CONNECTION DETAILS FOR CONTINUITY PLATE REQUIREMENTS.
7. BOLT EDGE DISTANCE, Leh SHALL BE EQUAL TO TWICE THE BOLT DIAMETER FOR BOTH THE PLATE AND THE BEAM WEB.
8. BOLT EDGE DISTANCE, Lev SHALL BE 1.1/4" FOR BOLT DIAMETERS 7/8" OR LESS AND 1.3/4xBOLT DIAMETER FOR BOLT DIAMETERS GREATER THAN 7/8".





2 BEAM CONNECTION SCHEDULE (7/8", HSSWF)

[1/S111A]

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DATE: APRIL 15, 2024

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1 04/15/24 ADDENDUM 1

BID SET - SECTION 1

SHEET TITLE:
SCHEDULES

SHEET NUMBER:
S602

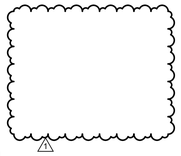
MASONRY WALL SCHEDULE

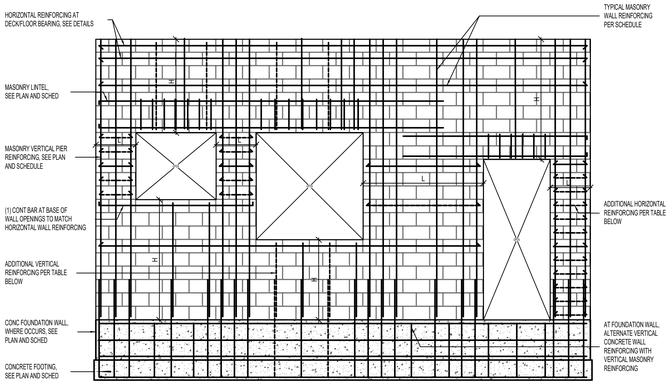
MARK	THICKNESS	MATERIAL	SOLID GROUT	TYPICAL REINFORCING (SEE NOTE 1)		COMMENTS
				VERTICAL	HORIZONTAL	
MW-8A	8"	SEE ARCH	YES	#5 AT 24" O.C.	#4 AT 24" O.C.	SEE NOTE 11
MW-8B	8"	SEE ARCH	YES	#4 AT 8" O.C.	#3 AT 8" O.C.	SEE NOTE 11
MW12A	12"	SEE ARCH	YES	#5 AT 8" O.C.	#4 AT 8" O.C.	SEE NOTE 11
MW-8C	8"	SEE ARCH	YES	#5 AT 32" O.C.	#4 AT 16" O.C.	SEE NOTE 11
MW12A	12"	SEE ARCH	YES	#5 AT 16" O.C.	#3 AT 8" O.C.	SEE NOTE 11

MASONRY WALLS NOT DESIGNATED IN PLAN

THICKNESS	REINFORCING		
	VERTICAL	HORIZONTAL (NOT SOLID GROUTED)	HORIZONTAL (SOLID GROUTED)
6"	#5 AT 32" O.C.	#4 AT 48" O.C.	#4 AT 34" O.C.
8"	#5 AT 32" O.C.	#5 AT 48" O.C.	#4 AT 24" O.C.
10"	#5 AT 24" O.C.	#6 AT 48" O.C.	#5 AT 34" O.C.
12"	#5 AT 24" O.C.	(2) #5 AT 48" O.C.	(2) #4 AT 24" O.C.

MASONRY WALL NOTES:
1. SPACING OF MASONRY WALL REINFORCING SHALL NOT EXCEED TYPICAL SCHEDULED REINFORCING. SEE ELEVATION AND MASONRY WALL SECTION REINFORCING TABLE BELOW FOR LOCATIONS WHERE TIGHTER SPACING IS REQUIRED.
2. COORDINATE WALL FINISHES, MATERIALS, COURSING, ETC. WITH ARCHITECTURAL DRAWINGS.
3. DO NOT SOLID GROUT WALLS UNLESS REQUIRED BY SCHEDULE, NOTES, OR DETAILS.
4. SOLID GROUT ALL MASONRY COURSES BELOW GRADE.
5. SINGLE LAYER OF VERTICAL REINFORCING SHALL BE CENTERED IN WALL (UNO).
6. VERTICAL REINFORCING SHALL EXTEND INTO FOOTINGS AND TERMINATE WITH STANDARD HOOK. FOR CONCRETE FOUNDATION WALLS 4'-0" OR TALLER, VERTICAL WALL REINFORCING SHALL DOWEL 3'-0" MINIMUM INTO THE FOUNDATION WALL (UNO).
7. PROVIDE TWO VERTICAL BARS (MIN) AT ALL CORNERS AND END OF WALLS.
8. HORIZONTAL WALL REINFORCING SHALL BE PLACED BETWEEN DOUBLE LAYER OF VERTICAL MASONRY REINFORCING, WHERE OCCURS.
9. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
10. SEE DETAILS 103501 FOR WHERE HORIZONTAL REINFORCING TERMINATES AT EDGE OF OPENINGS.
11. IN CONCRETE FOUNDATION WALL BELOW, ALTERNATE VERTICAL CONCRETE WALL REINFORCING WITH VERTICAL MASONRY REINFORCING.
12. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.





MARKS AND SYMBOLS LEGEND

———	INDICATES SCHEDULED MASONRY WALL, PIER, OR LINTEL REINFORCING
- - - - - - -	INDICATES ADDITIONAL REINFORCING AS REQUIRED PER MASONRY WALL SECTION REINFORCING TABLE
L	INDICATES LENGTH OF WALL SECTION
H	INDICATES HEIGHT OF WALL SECTION

MASONRY WALL SECTION REINFORCING TABLE

HEIGHT OR LENGTH	MAXIMUM SPACING
H OR L < 4'-0"	8" O.C.
4'-0" < H OR L < 6'-0"	16" O.C.
6'-0" < H OR L < 8'-0"	24" O.C.
8'-0" < H OR L < 10'-0"	32" O.C.
10'-0" < H OR L < 12'-0"	40" O.C.
H OR L > 12'-0"	48" O.C.

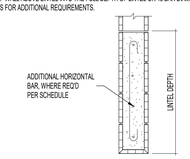
NOTES:
1. ADDITIONAL VERTICAL AND HORIZONTAL REINFORCING SHALL MATCH BAR SIZE OF SCHEDULED WALL REINFORCING AT SPACING INDICATED IN TABLE ABOVE.
2. WHERE 8" SPACING IS REQUIRED, #3 BAR MAY BE USED FOR HORIZONTAL REINFORCING.
3. WHERE SPACING OF SCHEDULED WALL REINFORCING IS LESS THAN TABLE ABOVE, SCHEDULED SPACING SHALL GOVERN.

1 MASONRY WALL SCHEDULE — NO SCALE

MASONRY LINTEL SCHEDULE

Mark	Depth	Max Span For Unscheduled Openings	Reinforcing		Comments
			Horizontal	Stirrups	
ML-16A	16"	4'-0"	(1) #6 x CONT TOP AND BOTTOM	NONE	
ML-24A	24"	4'-0"	(1) #6 x CONT TOP AND BOTTOM	#4 AT 8" O.C.	
ML-32A	32"	8'-0"	(1) #7 x CONT TOP AND BOTTOM	#4 AT 8" O.C.	
ML-40A	40"	8'-0"	(1) #7 x CONT TOP AND BOTTOM	#4 AT 8" O.C.	
ML-40B	40"	16'-0"	(5) #7 x CONT	#4 AT 8" O.C.	
ML-48A	48"	8'-0"	(1) #7 x CONT TOP AND BOTTOM	#4 AT 8" O.C.	
ML-56A	56"	14'-0"	(1) #7 x CONT TOP AND BOTTOM	#4 AT 8" O.C.	

MASONRY LINTEL NOTES:
1. LINTEL WIDTH AND MATERIAL TYPE SHALL BE THE SAME AS THE WALL IN WHICH THE LINTEL IS CONSTRUCTED.
2. GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR COLUMN AT EACH END.
3. MASONRY LINTELS ML-8A, ML-16A, ML-24A, AND ML-32A SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. WHEN A LINTEL IS SPECIFIED ON THE PLANS, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SPECIFIED ON THE PLANS WHICH HAVE A SPAN GREATER THAN 10'-0".
4. MASONRY LINTELS ML-8A, ML-16A, ML-24A, AND ML-32A SHALL NOT BE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR GIRDERS UNLESS NOTED OTHERWISE ON THE PLANS. JOISTS SHALL NOT BEAR ON ANY LINTEL LESS THAN 16" DEEP. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SHOWN ON THE PLANS WHICH ARE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR GIRDERS.
5. EXTEND ALL HORIZONTAL REINFORCING 48 BAR DIAMETERS MINIMUM BEYOND THE EDGE OF ALL OPENINGS. IF HORIZONTAL REINFORCING CANNOT EXTEND 48 BAR DIAMETERS BEYOND EDGE OF OPENING, PROVIDE 90° STANDARD HOOK.
6. SPLICE TOP BARS AT MIDSPAN OF LINTEL ONLY AND BOTTOM BARS OVER SUPPORTS ONLY.
7. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
8. DOWEL VERTICAL REINFORCING OF WALL ABOVE LINTEL INTO THE FULL DEPTH OF LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS.
9. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



2 MASONRY LINTEL SCHEDULE

MASONRY REINFORCING LAP SPLICE SCHEDULE

BAR SIZE	8" MASONRY		10" MASONRY		12" MASONRY	
	(1) BAR PER CELL	(2) BARS PER CELL	(1) BAR PER CELL	(2) BARS PER CELL	(1) BAR PER CELL	(2) BARS PER CELL
#3	12"	12"	12"	12"	12"	12"
#4	13"	21"	12"	20"	12"	20"
#5	20"	33"	16"	32"	13"	32"
#6	38"	SEE NOTE 1	29"	60"	24"	60"
#7	52"	SEE NOTE 1	40"	SEE NOTE 1	33"	63"
#8	SEE NOTE 1	SEE NOTE 1	61"	SEE NOTE 1	50"	SEE NOTE 1

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NOTES:
1. WHERE INDICATED, USE MECHANICAL SPLICE COUPLER. SEE GSN FOR REQUIREMENTS.
2. WHERE VERTICAL BARS HAVE A SPECIFIED LAP SPLICE GREATER THAN THE HEIGHT OF THE GROUT POUR, USE MECHANICAL SPLICE COUPLER.

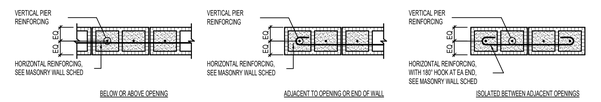
3 MASONRY REINFORCING LAP SPLICE SCHEDULE (f'm=2000psi) — NO SCALE

MASONRY PIER SCHEDULE

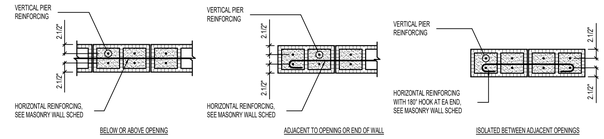
MARK	SIZE	VERTICAL REINFORCING	VERTICAL REINFORCING SCHEMATIC	COMMENTS
MP-16A	WT x 16"	(2) #5		
MP-16B	WT x 16"	(4) #5		
MP-24A	WT x 24"	(3) #5		
MP-32A	WT x 32"	(4) #5		
MP-32B	WT x 32"	(8) #5		
MP-40B	WT x 40"	(10) #5		
MP-48B	WT x 48"	(12) #5		
MP-56A	WT x 56"	(7) #4		
MP-64A	WT x 64"	(8) #5		
MP-64B	WT x 64"	(16) #5		

MASONRY PIER NOTES:
1. SEE MASONRY WALL SCHEDULE FOR HORIZONTAL REINFORCING REQUIREMENTS FOR ALL PIERS.
2. VERTICAL REINFORCING AND TIES SHALL EXTEND FULL HEIGHT OF WALL (UNO).
3. VERTICAL MASONRY PIER REINFORCING SHALL EXTEND INTO THE FOOTING AND TERMINATE WITH A STANDARD 90° HOOK. FOR CONCRETE FOUNDATION WALLS 4'-0" OR TALLER, VERTICAL PIER REINFORCING SHALL DOWEL 3'-0" MINIMUM INTO THE FOUNDATION WALL (UNO).
4. IN CONCRETE FOUNDATION WALLS, VERTICAL REINFORCING AT TYPE 'B' MASONRY PIERS SHALL BE TIED WITH #3 TIES AT TOP AND BOTTOM OF FOUNDATION WALL. SEE DETAILS.
5. HORIZONTAL REINFORCING OF ADJACENT WALLS SHALL RUN CONTINUOUS THROUGH MASONRY PIERS.
6. WHERE HORIZONTAL REINFORCING TERMINATES AT PIER, PROVIDE 180° HOOK, SEE SCHEMATICS BELOW.
7. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
8. PROVIDE #3 TIES AT 8" O.C. WHERE MASONRY PIERS SUPPORT GIRDER TRUSSES.

TYPE A PIER CONFIGURATION SCHEMATICS



TYPE B PIER CONFIGURATION SCHEMATICS

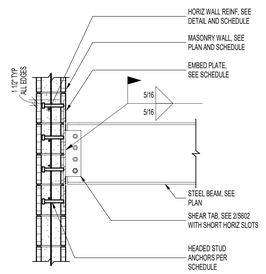


4 MASONRY PIER SCHEDULE — NO SCALE

CONNECTION SCHEDULE

BEAM DEPTH	EMBED PLATE	ANCHORS
W10	PL 1/2"x16"x1'-0"	(3) ROWS OF (2) 3/4"DIA x 5" HSA ((6) TOTAL)
W12, W14	PL 1/2"x21"x1'-0"	(4) ROWS OF (2) 3/4"DIA x 5" HSA ((8) TOTAL)
W16	PL 1/2"x27"x1'-0"	(5) ROWS OF (2) 3/4"DIA x 5" HSA ((10) TOTAL)
W18	PL 1/2"x27"x1'-9"	(5) ROWS OF (3) 3/4"DIA x 5" HSA ((15) TOTAL)
W21	PL 1/2"x33"x1'-9"	(6) ROWS OF (3) 3/4"DIA x 5" HSA ((18) TOTAL)

CONNECTION NOTES:
1. ALL MASONRY CELLS WITH ANCHORS AND ADJACENT TO ANCHORS SHALL BE GROUTED SOLID, TYP.



5 TYPICAL EMBED PLATE CONNECTION SCHEDULE FOR MASONRY WALLS
3/4" = 1'-0"

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: OPERATIONS BUILDING - SECTION 1

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

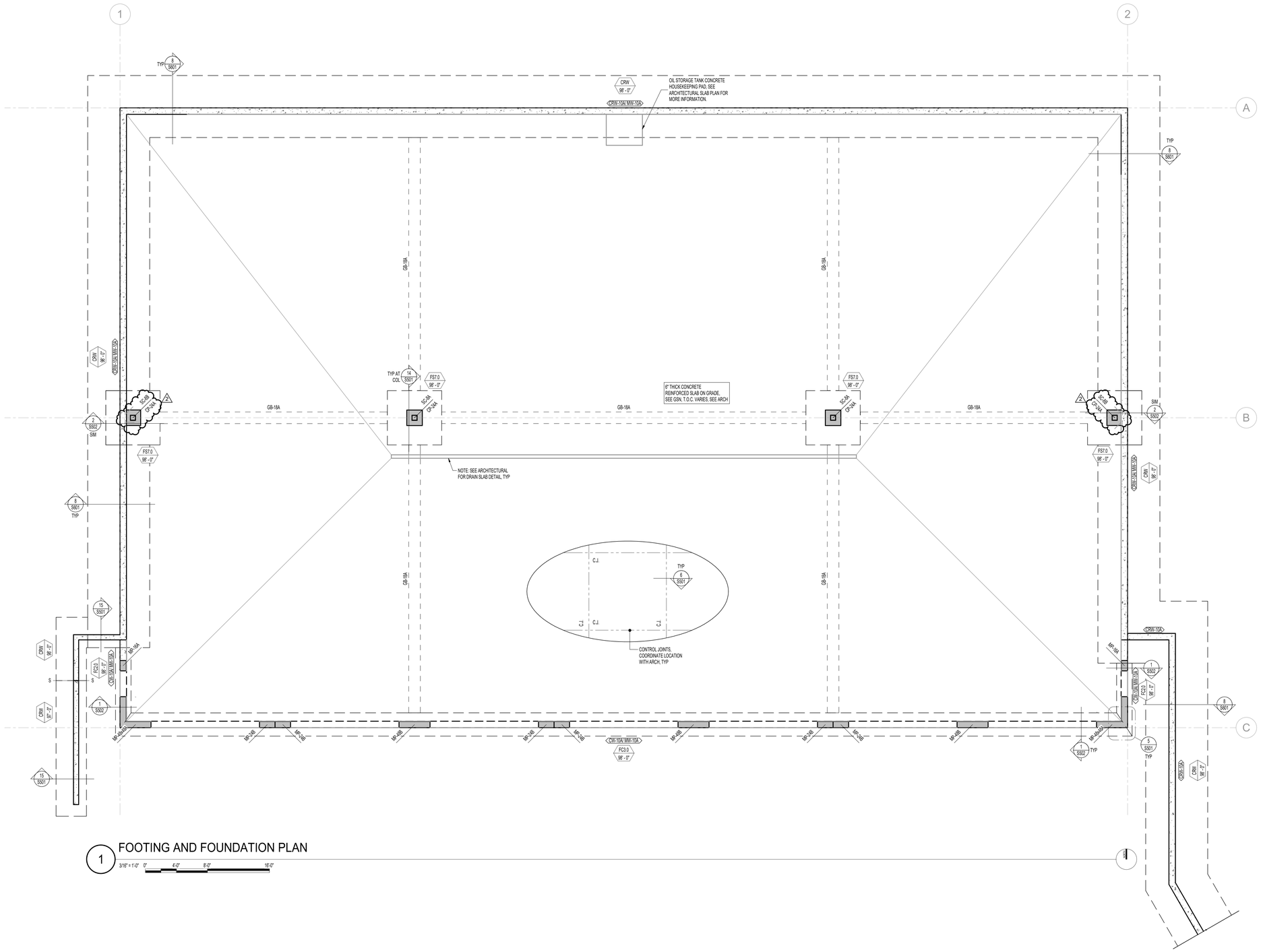
BHB PROJECT NO. 230074
DATE: APRIL 15, 2024

REVISIONS:
1 04/15/24 ADDENDUM 1

BID SET - SECTION 1

SHEET TITLE:
SCHEDULES

SHEET NUMBER:
S603



1 FOOTING AND FOUNDATION PLAN

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

FOOTING AND FOUNDATION PLAN NOTES

1. COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
3. SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LOCATIONS.
4. ALL SPOT FOOTINGS SHALL BE CENTERED UNDER COLUMNS (UNO).
5. SEE DETAILS 1/S501 AND 2/S501 FOR CONDITION WHERE BURIED PIPES RUN PARALLEL AND PERPENDICULAR TO FOOTINGS.
6. SEE DETAIL 6/S501 FOR TYPICAL CONTROL/CONSTRUCTION JOINTS IN CONCRETE SLAB ON GRADE.
7. SEE DETAIL 8/S501 FOR SLAB REINFORCING WHERE CONTROL JOINTS ARE DISCONTINUOUS.
8. SEE DETAIL 9/S501 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN CONCRETE WALLS.
9. SEE DETAIL 10/S501 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
10. SEE DETAIL 11/S501 FOR CONDITION AT RECESSES IN MASONRY WALLS.
11. SEE DETAIL 12/S501 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS
12. SEE DETAIL 13/S501 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
13. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: VEHICULAR STORAGE BUILDING - SECTION 2

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BHB PROJECT NO. 230674
DATE: APRIL 15, 2024

REVISIONS:

2 04/15/24 ADDENDUM 1

BID SET - SECTION 2

SHEET TITLE:
FOOTING AND FOUNDATION PLAN

SHEET NUMBER:
S101

Plot Date: 4/15/2024 2:59:37 PM

CONCRETE CONTINUOUS FOOTING SCHEDULE (FC)

MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE				REINFORCING LENGTHWISE				COMMENTS
				No.	SIZE	LENGTH	SPACING	No.	SIZE	LENGTH	SPACING	
FC2.0	2' - 0"	CONT	12"	-	#4	1' - 6"	48"	3	#4	CONT	EQ	
FC3.0	3' - 0"	CONT	18"	-	#6	2' - 6"	12"	3	#6	CONT	EQ	

CONCRETE SPOT FOOTING SCHEDULE (FS)

MARK	WIDTH	Length	DEPTH	REINFORCING CROSSWISE				REINFORCING LENGTHWISE				COMMENTS
				No.	SIZE	LENGTH	SPACING	No.	SIZE	LENGTH	SPACING	
FS7.0	7' - 0"	7' - 0"	18"	7	#6	6' - 6"	EQ	7	#6	6' - 6"	EQ	

CONCRETE FOOTING NOTES:
1. PLACE ALL FOOTING REINFORCING IN THE BOTTOM OF THE FOOTING WITH 3" CLEAR CONCRETE COVER (UNO).
2. TOP REINFORCING, WHERE OCCURS, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
3. IF FOOTINGS ARE EARTH-FORMED, FOOTINGS SHALL BE 6" LONGER AND WIDER THAN SCHEDULED.
4. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
5. SOME SCHEDULED FOOTINGS MAY NOT BE USED, SEE FOOTING AND FOUNDATION PLAN FOR FOOTING MARKS.

1 CONCRETE FOOTING SCHEDULE (C3000-S2000)
1/2" = 1'-0"

CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE

BAR SIZE	f'c = 3000psi & f'c = 3500 psi				f'c = 4000psi & f'c = 4500 psi				f'c = 5000psi				f'c = 6000psi			
	REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP	
	CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS	
	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B
#3	17"	22"	22"	28"	15"	19"	19"	24"	13"	17"	17"	22"	12"	16"	15"	20"
#4	22"	29"	29"	37"	19"	25"	25"	32"	17"	22"	22"	29"	16"	20"	20"	27"
#5	28"	36"	36"	47"	24"	31"	31"	40"	22"	28"	28"	36"	20"	26"	26"	33"
#6	33"	43"	43"	56"	29"	37"	37"	48"	26"	33"	33"	43"	24"	31"	31"	40"
#7	48"	63"	63"	81"	42"	54"	54"	70"	37"	49"	49"	63"	34"	44"	44"	58"
#8	55"	72"	72"	93"	48"	62"	62"	80"	43"	56"	55"	72"	39"	51"	51"	66"
#9	62"	81"	81"	105"	54"	70"	70"	91"	48"	63"	63"	81"	44"	57"	57"	74"
#10	70"	91"	91"	118"	61"	79"	79"	102"	54"	70"	70"	91"	50"	64"	64"	83"
#11	78"	101"	101"	131"	67"	87"	87"	113"	60"	78"	78"	101"	55"	71"	71"	93"

TABULATED VALUES ARE FOR CASE 1 REINFORCEMENT, WHERE THE REQUIREMENTS OF TABLE BELOW ARE MET. WHERE THESE CONDITIONS ARE NOT MET, MULTIPLY THE LAP LENGTHS (ℓd) BY 1.5.

REQUIREMENT FOR CASE 1 LAP LENGTHS

db = BAR DIAMETER

BAR CLEAR SPACING	CLEAR COVER	STIRRUPS OR TIES
>=db	>=db	>=CODE FOR MINIMUM THROUGHOUT ℓd
>=2db	>=db	NO REQUIREMENT

CONCRETE REINFORCING BAR LAP SPLICE NOTES:
1. THIS SCHEDULE SHALL BE USED FOR ALL BAR SPLICES IN CONCRETE WALLS, UNLESS NOTED OTHERWISE.
2. CLASS 'A' SPLICES MAY BE USED ONLY IN CASES WHERE 50% OR LESS OF THE BARS ARE SPLICED WITHIN THE LAP SPLICE LENGTH.
3. CLASS 'B' SPLICES SHALL BE USED FOR ALL SPLICES UNLESS THE REQUIREMENTS OF NOTE No. 2 ABOVE ARE MET.
4. TIES AND STIRRUPS SHALL NOT BE SPLICED.
5. DO NOT SPLICE VERTICAL BARS IN RETAINING WALLS UNLESS SPECIFICALLY SHOWN.
6. THE VALUES TABULATED IN SCHEDULE ARE FOR GRADE 60 REINFORCING BARS. FOR GRADE 75, MULTIPLY LAP LENGTHS BY 1.25 AND FOR GRADE 80, MULTIPLY BY 1.33.
7. THE VALUES TABULATED IN SCHEDULE ARE MINIMUM REQUIREMENTS. LONGER LENGTHS MAY BE USED FOR CONSTRUCTIBILITY.
8. LAP SPLICES ARE NOT ALLOWED FOR BARS GREATER THAN #11 BAR. THE LENGTHS IN SCHEDULE ARE FOR TENSION DEVELOPMENT LENGTH.
9. TOP BARS ARE CLASSIFIED AS HORIZONTAL BARS WHERE 12", OR MORE, OF FRESH CONCRETE IS CAST BELOW THE REINFORCING BAR.
10. FOR EPOXY-COATED OR ZINC AND EPOXY DUAL-COATED BARS WITH CLEAR COVER < 3db OR CLEAR SPACING <6db , MULTIPLY LAP LENGTHS BY 1.5. FOR ALL OTHER CASES MULTIPLY BY 1.2
11. SPLICES FOR BUNDLED BARS:
    A. FOR BUNDLED BARS OF THREE OR LESS, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.2.
    B. FOR BUNDLED BARS OF FOUR OR MORE, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.33.
    C. INDIVIDUAL BAR SPLICES WITHIN A BUNDLE SHALL NOT OVERLAP.
    D. ENTIRE BUNDLES SHALL NOT BE LAP SPLICED.
12. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

2 CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE
3/4" = 1'-0"

CONCRETE WALL SCHEDULES

MARK	THICKNESS	REINFORCING				COMMENTS
		VERTICAL	HORIZONTAL	TOP AND BOTTOM	WALL TYPE	
CW-10A	10"	#4 AT 24" O.C.	#5 AT 15" O.C.	#5	A	SEE NOTE 2

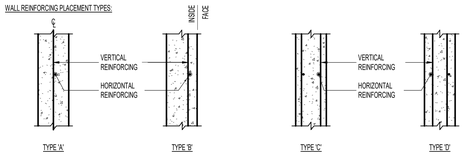
CONCRETE FOUNDATION WALL NOTES:
1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
2. STAGGER CONCRETE VERTICAL REINFORCING WITH MASONRY VERTICAL DOWELS. NET SPACING OF VERTICAL REINFORCING SHALL BE SPECIFIED SPACING DIVIDED BY HALF.

ABBREVIATIONS:
EACH FACE E.F.
INSIDE FACE I.F.
OUTSIDE FACE O.F.

WALLS NOT DESIGNATED IN PLAN

THICKNESS	REINFORCING	
	VERTICAL	HORIZONTAL
6"	#4 BARS AT 18" O.C.	#4 BARS AT 16" O.C.
8"	#4 BARS AT 18" O.C.	#4 BARS AT 12" O.C.
10"	#4 BARS AT 16" O.C.	#5 BARS AT 15" O.C.
12"	#4 BARS AT 18" O.C. E.F.	#4 BARS AT 16" O.C. E.F.

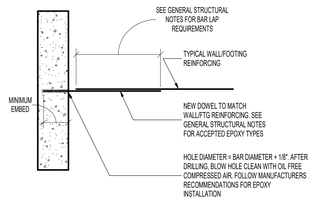
WALL REINFORCING PLACEMENT TYPES:



3 CONCRETE WALL SCHEDULE
3/4" = 1'-0"

EPOXY DOWEL EMBED SCHEDULE

Dowel Size	Min Embedment Into Existing Concrete
#4	6 1/2"
#5	7 1/2"
#6	10"
#7	1'-1"
#8	1'-4"



4 EPOXY DOWEL EMBED SCHEDULE
3/4" = 1'-0"

STEEL COLUMN SCHEDULE

MARK	STEEL COLUMN - TYPE: SIZE	STEEL BASE PLATE		STEEL CAP PLATE		COMMENTS
		THICKNESS	PLATE TYPE	THICKNESS	PLATE TYPE	
SC-8A	SC - HSS : HSS6X6X5/16	3/4"	A	1/2"	A	
SC-8B	SC - HSS : HSS6X6X1/4 (2)	3/4"	B	1/2"	B	

STEEL COLUMN NOTES:
1. UNLESS NOTED OTHERWISE, ALL COLUMNS SHALL BE INSTALLED WITH (4) 3/4"DIA ANCHOR RODS WITH 3" MINIMUM HOOKS. PROJECT ANCHOR RODS 3" MINIMUM ABOVE THE TOP OF THE BASE PLATE. EMBEDMENT SHALL BE 9" MINIMUM. ALL RODS SHALL BE INSTALLED WITH HARDENED WASHERS BENEATH THE NUT. ANY BOLT HOLES LARGER THAN THE ROD DIAMETER PLUS 5/16" SHALL HAVE 5/16" PLATE WASHERS INSTALLED BENEATH THE HARDENED WASHERS.
2. ALL CAP PLATE BOLTS SHALL BE 3/4"DIA A325N BOLTS. TYPICAL UNLESS NOTED OTHERWISE.
3. ANCHOR RODS SHALL NOT BE WELDED (INCLUDING TACK WELDS).
4. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

STEEL BASE PLATE TYPES:

PLATE LEGEND
cp = 1'2" MINIMUM
ed = 1 1/2" MINIMUM
sp = 3" MINIMUM
bc = BEAM OR GIRDER GAGE
w = BEAM OR GIRDER GAGE +3"
OR
BEAM OR GIRDER WIDTH +1"
OR
COLUMN WIDTH + 1"
WHICHEVER IS GREATER

STEEL CAP PLATE TYPES:

5 STEEL COLUMN SCHEDULE
3/4" = 1'-0"

CONCRETE GRADE BEAM SCHEDULE

MARK	BEAM SIZE H x W	REINFORCING			COMMENTS
		LENGTHWISE	TIES	TYPE	
GB-18A	18" x 18"	(6) #6	(3) #3 TIES AT 6" O.C.	A	

CONCRETE GRADE BEAM NOTES:
1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

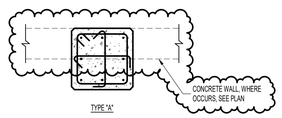
TYPE 'A'

6 CONCRETE GRADE BEAM SCHEDULE
3/4" = 1'-0"

CONCRETE PIER SCHEDULE

MARK	PIER SIZE W x L	REINFORCING		TYPE	COMMENTS
		VERTICAL	TIES		
CP-24A	24" x 24"	(8) #6	(3) #3 AT 8" O.C.	A	

CONCRETE PIER NOTES:
1. INSTALL (3) SETS OF TIES WITHIN TOP 5" OF ALL PIERS (UNO).
2. RUN HORIZONTAL CONCRETE WALL REINFORCING CONTINUOUS THROUGH PIER WHEN PIER IS POURED MONOLITHICALLY WITH CONCRETE WALL.
3. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



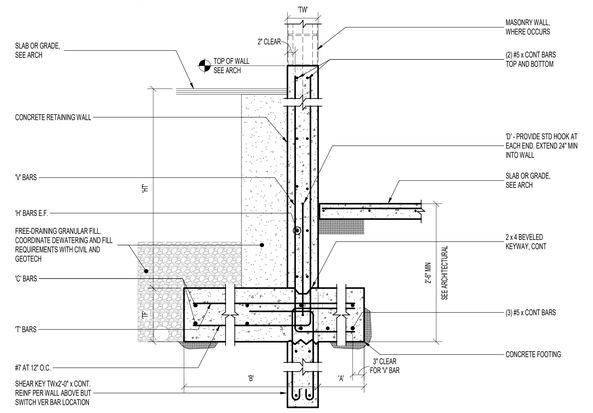
TYPE 'A'

7 CONCRETE PIER SCHEDULE
3/4" = 1'-0"

CRW FOOTING / CONCRETE RETAINING WALL SCHEDULE

MARK	DIMS 'A'	DIMS TF	DIMS 'B'	HT	TW	V SIZE	V SPACE	H SIZE	H SPACE	T SIZE	T SPACE	C SIZE	C SPACE	D SIZE	D SPACE	COMMENTS
CRW-10A	3'-0"	18"	5'-0"	9' - 4"	10"	#7	10"	#5	12"	#7	14"	#5	12"	#7	12"	

CONCRETE RETAINING WALL NOTES:
1. 'V' BARS SHALL NOT BE SPLICED BELOW MID-HEIGHT OF WALL.
2. PROVIDE VERTICAL CONTRACTION JOINTS AT 20'-0" O.C. MAXIMUM. SEE ARCHITECTURAL DRAWINGS AND GENERAL STRUCTURAL NOTES.
3. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



8 CONCRETE RETAINING WALL SCHEDULE
3/4" = 1'-0"
(1/S101)

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: VEHICULAR STORAGE BUILDING - SECTION 2

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BHB PROJECT NO. 230674
DATE: APRIL 15, 2024

REVISIONS:

2 04/15/24 ADDENDUM 1

BID SET - SECTION 2

SHEET TITLE:
SCHEDULES

SHEET NUMBER:
S601

Plot Date: 4/15/2024 2:59:41 PM

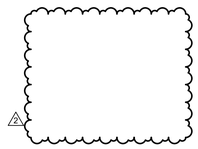
MASONRY WALL SCHEDULES

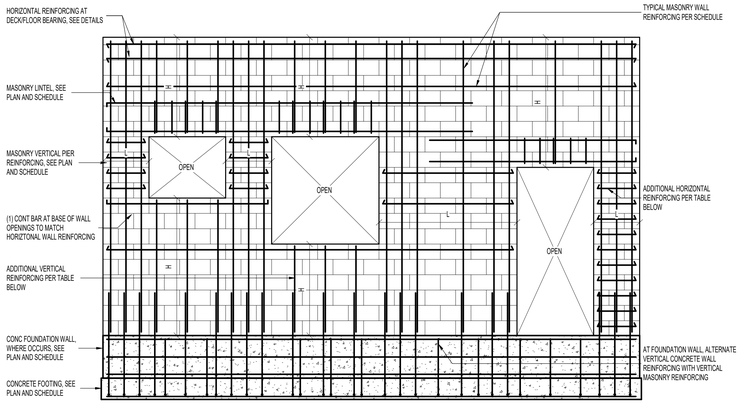
MARK	THICKNESS	MATERIAL	SOLID GROUT	TYPICAL REINFORCING (SEE NOTE 1)		COMMENTS
				VERTICAL	HORIZONTAL	
MW-10A	10"	CMU	No	#5 AT 24" O.C.	#6 AT 48" O.C.	

MASONRY WALLS NOT DESIGNATED IN PLAN

THICKNESS	REINFORCING		
	VERTICAL	HORIZONTAL (NOT SOLID GROUTED)	HORIZONTAL (SOLID GROUTED)
6"	#5 AT 32" O.C.	#4 AT 48" O.C.	#4 AT 24" O.C.
8"	#5 AT 32" O.C.	#5 AT 48" O.C.	#4 AT 24" O.C.
10"	#5 AT 24" O.C.	#6 AT 48" O.C.	#5 AT 24" O.C.
12"	#5 AT 24" O.C.	(2) #5 AT 48" O.C.	(2) #4 AT 24" O.C.

MASONRY WALL NOTES:
1. SPACING OF MASONRY WALL REINFORCING SHALL NOT EXCEED TYPICAL SCHEDULED REINFORCING. SEE ELEVATION AND MASONRY WALL SECTION REINFORCING TABLE BELOW FOR LOCATIONS WHERE TIGHTER SPACING IS REQUIRED.
2. COORDINATE WALL FINISHES, MATERIALS, COURSING, ETC. WITH ARCHITECTURAL DRAWINGS.
3. DO NOT SOLID GROUT WALLS UNLESS REQUIRED BY SCHEDULE, NOTES, OR DETAILS.
4. SOLID GROUT ALL MASONRY COURSES BELOW GRADE.
5. SINGLE LAYER OF VERTICAL REINFORCING SHALL BE CENTERED IN WALL (UNO).
6. VERTICAL REINFORCING SHALL EXTEND INTO FOOTINGS AND TERMINATE WITH STANDARD HOOK. FOR CONCRETE FOUNDATION WALLS 4'-0" OR TALLER, VERTICAL WALL REINFORCING SHALL DOWEL 3'-0" MINIMUM INTO THE FOUNDATION WALL (UNO).
7. PROVIDE TWO VERTICAL BARS (MIN) AT ALL CORNERS AND END OF WALLS.
8. HORIZONTAL WALL REINFORCING SHALL BE PLACED BETWEEN DOUBLE LAYER OF VERTICAL MASONRY REINFORCING, WHERE OCCURS.
9. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
10. SEE DETAIL 13S501 FOR WHERE HORIZONTAL REINFORCING TERMINATES AT EDGE OF OPENINGS.
11. IN CONCRETE FOUNDATION WALL BELOW, ALTERNATE VERTICAL CONCRETE WALL REINFORCING WITH VERTICAL MASONRY REINFORCING.
12. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.





MARKS AND SYMBOLS LEGEND

———	INDICATES SCHEDULED MASONRY WALL, PIER, OR LINTEL REINFORCING
- - - -	INDICATES ADDITIONAL REINFORCING AS REQUIRED PER MASONRY WALL SECTION REINFORCING TABLE
L	INDICATES LENGTH OF WALL SECTION
H	INDICATES HEIGHT OF WALL SECTION

MASONRY WALL SECTION REINFORCING TABLE

HEIGHT OR LENGTH	MAXIMUM SPACING
H OR L ≤ 4'-0"	8" O.C.
4'-0" < H OR L ≤ 6'-0"	16" O.C.
6'-0" < H OR L ≤ 8'-0"	24" O.C.
8'-0" < H OR L ≤ 10'-0"	32" O.C.
10'-0" < H OR L ≤ 12'-0"	40" O.C.
H OR L > 12'-0"	48" O.C.

NOTES:
1. ADDITIONAL VERTICAL AND HORIZONTAL REINFORCING SHALL MATCH BAR SIZE OF SCHEDULED WALL REINFORCING AT SPACING INDICATED IN TABLE ABOVE.
2. WHERE 8" SPACING IS REQUIRED, #3 BAR MAY BE USED FOR HORIZONTAL REINFORCING.
3. WHERE SPACING OF SCHEDULED WALL REINFORCING IS LESS THAN TABLE ABOVE, SCHEDULED SPACING SHALL GOVERN.

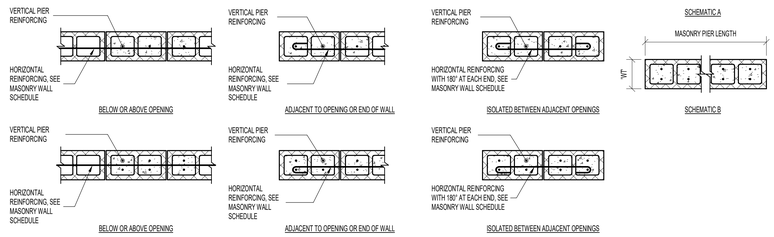
1 MASONRY WALL SCHEDULE
3/4" = 1'-0"

MASONRY PIER SCHEDULE

MARK	PIER SIZE 'WT' x L		VERTICAL REINFORCING	TYPE	COMMENTS
MP-16A	16"	16"	(2) #5	A	
MP-24B	24"	24"	(4) #5	B	
MP-48B	48"	48"	(8) #5	B	
MP-48x48A	48"	48"			

MASONRY PIER NOTES:
1. SEE MASONRY WALL SCHEDULE FOR HORIZONTAL REINFORCING REQUIREMENTS FOR ALL PIERS.
2. VERTICAL REINFORCING AND TIES SHALL EXTEND FULL HEIGHT OF WALL (UNO).
3. VERTICAL MASONRY PIER REINFORCING SHALL EXTEND INTO THE FOOTING AND TERMINATE WITH A STANDARD 90° HOOK. FOR CONCRETE FOUNDATION WALLS 4'-0" OR TALLER, VERTICAL PIER REINFORCING SHALL DOWEL 3'-0" MINIMUM INTO THE FOUNDATION WALL (UNO).
4. IN CONCRETE FOUNDATION WALLS, VERTICAL REINFORCING AT TYPE 'B' MASONRY PIERS SHALL BE TIED WITH #3 TIES AT TOP AND BOTTOM OF FOUNDATION WALL, SEE DETAILS.
5. HORIZONTAL REINFORCING OF ADJACENT WALLS SHALL RUN CONTINUOUS THROUGH MASONRY PIERS.
6. WHERE HORIZONTAL REINFORCING TERMINATES AT PIER, PROVIDE 180° HOOK, SEE SCHEMATICS BELOW.
7. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

TYPE A PIER CONFIGURATION SCHEMATICS



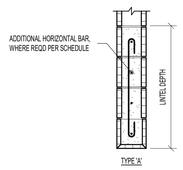
MASONRY SCHEMATICS:

2 MASONRY PIER SCHEDULE
3/4" = 1'-0"

MASONRY LINTEL SCHEDULE NEW

MARK	LINTEL DEPTH	REINFORCING		TYPE	COMMENTS
		HORIZONTAL	STIRRUPS		
ML-16A	16"	(1) #5 x CONT TOP AND BOTTOM	NONE	A	
ML-48A	48"	(1) #7 x CONT TOP AND BOTTOM	#4 AT 8" O.C.	A	

MASONRY LINTEL NOTES:
1. LINTEL WIDTH AND MATERIAL TYPE SHALL BE THE SAME AS THE WALL IN WHICH THE LINTEL IS CONSTRUCTED.
2. GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR PIER AT EACH END.
3. MASONRY LINTEL ML-9A SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. WHEN A LINTEL IS SPECIFIED ON THE PLANS, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SPECIFIED ON THE PLANS WHICH HAVE A SPAN GREATERTHAN 3'-4".
4. MASONRY LINTEL ML-9A SHALL NOT BE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR GIRDERS UNLESS NOTED OTHERWISE ON THE PLANS. JOISTS SHALL NOT BEAR ON ANY LINTEL LESS THAN 10" DEEP. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SHOWN ON THE PLANS WHICH ARE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR GIRDERS.
5. EXTEND ALL HORIZONTAL REINFORCING 48 BAR DIAMETERS MINIMUM BEYOND THE EDGE OF ALL OPENINGS. IF HORIZONTAL REINFORCING CANNOT EXTEND 48 BAR DIAMETERS BEYOND EDGE OF OPENING, PROVIDE 90° STANDARD HOOK.
6. SPLICE TOP BARS AT MIDSPAN OF LINTEL ONLY AND BOTTOM BARS OVER SUPPORTS ONLY.
7. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
8. DOWEL VERTICAL REINFORCING OF WALL ABOVE LINTEL INTO THE FULL DEPTH OF LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS.
9. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



3 MASONRY LINTEL SCHEDULE
3/4" = 1'-0"

MASONRY REINFORCING LAP SPLICE SCHEDULE

	8" MASONRY		10" MASONRY		12" MASONRY	
BAR SIZE	(1) BAR PER CELL	(2) BARS PER CELL	(1) BAR PER CELL	(2) BARS PER CELL	(1) BAR PER CELL	(2) BARS PER CELL
#3	12"	12"	12"	12"	12"	12"
#4	13"	21"	12"	20"	12"	20"
#5	20"	35"	16"	32"	13"	32"
#6	38"	SEE NOTE 1	29"	60"	24"	60"
#7	52"	SEE NOTE 1	40"	SEE NOTE 1	33"	63"
#8	SEE NOTE 1	SEE NOTE 1	51"	SEE NOTE 1	50"	SEE NOTE 1

2018 IBC

NOTES:
1. WHERE INDICATED, USE MECHANICAL SPLICE COUPLER. SEE GSN FOR REQUIREMENTS.
2. WHERE VERTICAL BARS HAVE A SPECIFIED LAP SPLICE GREATER THAN THE HEIGHT OF THE GROUT POUR, USE MECHANICAL SPLICE COUPLER.

4 MASONRY REINFORCING LAP SPLICE SCHEDULE (f'm=2000psi)
3/4" = 1'-0"

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CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT: VEHICULAR STORAGE BUILDING - SECTION 2

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

BHB PROJECT NO. 230674
DATE: APRIL 15, 2024

REVISIONS:
2 04/15/24 ADDENDUM 1

BID SET - SECTION 2

SHEET TITLE:
SCHEDULES

SHEET NUMBER:
S602

Plot Date: 4/15/2024 2:59:42 PM

NEW PAVEMENT LEGEND

NEW HOT MIX ASPHALT	
NEW CONCRETE FLATWORK	
24" STANDARD CURB AND GUTTER	
24" RELEASE CURB AND GUTTER	
24" TRANSITION CURB AND GUTTER	

SITE NOTES

1. FOR UTILITY INFORMATION SEE OVERALL UTILITY SITE PLAN.

SITE PLAN KEY NOTES

1. NEW OPERATIONS BUILDING (23,640 SF) – SEE GRADING AND UTILITY SHEETS.
2. NEW VEHICLE STORAGE BUILDING (10,400 SF) – SEE GRADING AND UTILITY SHEETS.
3. NEW PARKING LOT EXPANSION (41 NEW STALLS) (SEE SHEET C5.1 FOR GRADING PLAN)
4. NEW DECANT STRUCTURE. SEE GRADING PLAN FOR ELEVATIONS. FOR CONSTRUCTION SEE STRUCTURAL DETAIL SHEETS.
5. NEW MATERIAL STORAGE STRUCTURE. SEE GRADING PLAN FOR ELEVATIONS. FOR CONSTRUCTION DRAWINGS SEE STRUCTURAL SHEETS.
6. FUTURE ELECTRIC VEHICLE CHARGING STATIONS. SEE ARCHITECTURAL PLANS. (10 NEW PARKING STALLS)
7. NEW 24" CURB AND GUTTER PER APWA PLAN 205.1 (TYPE A) SEE DETAIL THIS SHEET
8. NEW 5' WIDE CONCRETE CROSS DRAIN. (SEE GRADING PLANS C5.2 & C5.4) AND DETAIL THIS SHEET.
9. FUEL STATION ISLAND STEPPED RETAINING WALL. SEE GRADING PLAN FOR ELEVATIONS. FOR CONSTRUCTION DRAWINGS SEE STRUCTURAL SHEETS.
10. NEW BUILDING RETAINING WING WALL WITH RAILING. SEE ARCHITECTURAL PLANS. FINISHED GRADE OUTSIDE OF BUILDING ARE SHOWN ON GRADING SHEETS.
11. NEW GRAVEL BETWEEN EXISTING FENCE AND PROPOSED ASPHALT.
12. NEW AUTOMATED GATE SYSTEM. CONTRACTOR TO COORDINATE WITH CLEARFIELD CITY FOR GATE & OPENER SELECTION. CONTRACTOR TO COORDINATE POWER AND INTERNET CONNECTION WITH SITE ARCHITECT.
13. 4" PARKING STALL STRIPING (YELLOW), TYP.
14. EXISTING PARK'S CONCRETE STORAGE STRUCTURE TO REMAIN.
15. EXISTING CONCRETE TO REMAIN.
16. EXISTING ELECTRICAL BOXES TO REMAIN.
17. NEW YARD LIGHT
18. NEW LANDSCAPE ISLANDS. INCLUDING LANDSCAPE CURB, TREES, BUSHES, NEW IRRIGATION AND DECORATE ROCK. SEE LANDSCAPE PLAN.
19. NEW ADA CONCRETE WALK WAY. SEE GRADING PLAN SHEET C5.1
20. EXISTING WATER METER/ VALVE BOX. RAISE LID TO PROPOSED GRADE.
21. NEW CURB ISLAND & CONCRETE FLAT WORK. SEE GRADING PLAN C5.1
22. NEW 3.0' MAN GATE. CONTRACTOR TO COORDINATE WITH CLEARFIELD CITY FOR TYPE.
23. NEW GATE OPENER PEDESTAL FOR GATE ACCESS. CONTRACTOR TO COORDINATE WITH CITY FOR THE INSTALL.
24. NEW LOCATION OF POLE SALT TRUCK BED HANGER. SEE STRUCTURAL PLANS FOR BOLTED ANCHOR AND THICKENED SLAB DETAILS. SEE GRADING PLAN FOR ELEVATIONS.
25. NEW DUMPSTER AREA – DUMPSTERS TO BE RELOCATED AFTER CONSTRUCTION.
26. NEW 6" THICK CONCRETE APRON WITH WELDED WIRE FABRIC REINFORCEMENT – SEE DETAIL THIS SHEET
27. NEW 5.0' SIDEWALK ADA RAMP PER CLEARFIELD CITY STANDARDS.
28. NEW PEDESTRIAN CROSSWALK PAINT
29. NEW VAN ACCESSIBLE ADA STALL WITH PAINT MARKINGS

0 20 40 60
Scale in Feet

ASPHALT SECTION

REINFORCED CONCRETE APRON SECTION

CROSS DRAIN SECTION

24" CURB AND GUTTER SECTION

24" RELEASE CURB AND GUTTER SECTION

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MARCH, 21 2024

CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT:

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

PROJECT NO. ----
DATE: MARCH, 21 2024

REVISIONS:
1. ADA SIDEWALK & STALL
2.
3.
4.
5.
6.
7.

SHEET TITLE:
SITE PLAN

SHEET NUMBER:
C4.0

CIVIL

JA

0 10 20 30

Scale in Feet

SEE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR RETAINING WALL INFORMATION, TYPICAL.

WEST SIDE RETAINING WALL SEE ARCHITECTURAL PLAN

EXISTING RETAINING WALL CUT AS SHOWN.

THREE RISERS.

SEE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR RETAINING WALL INFORMATION, TYPICAL.

STREET

CIVIL DRAWINGS #

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MARCH, 21 2024

CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

PROJECT NO. ----
DATE: MARCH, 21 2024

REVISIONS:
1. ADA SIDEWALK & STALL
2.
3.
4.
5.
6.
7.

SHEET TITLE:
GRADING PLAN

SHEET NUMBER:
C5.1

CIVIL

LEGEND

- NEW HOT MIX ASPHALT
- NEW CONCRETE FLATWORK
- 30" STANDARD CURB AND GUTTER
- 30" REVERSE PAN CURB AND GUTTER
- NEW ROCK/COBBLE AREAS
- LANDSCAPE BOUDARY
- 6" FLAT CONCRETE LANDSCAPE CURB

SITE TABULATION

1. SITE AREA =	271,462 SF
2. ROOF AREA =	59,973 SF
3. HARD SURFACE AREA =	146,035 SF
4. TURF LANDSCAPE AREA=	9,541 SF
5. GRAVEL LANDSCAPE AREA =	7,330 SF
6. TOTAL LANDSCAPE AREA =	16,871 SF
7. PERCENT LANDSCAPE AREA =	6.2 %

8. LANDSCAPE DENSITY
   - 1 TREE FOR EVERY 1000 SF OF LANDSCAPED AREA
   - 1 SHRUB FOR EVERY 600 SF OF LANDSCAPED AREA
9. TREES REQUIRED (17) TREES PROVIDED (17)
10. SHRUBS REQUIRED (28) SHRUBS PROVIDED (29)

IRRIGATION PLAN:

1) CONTRACTOR TO ASSES EXISTING SPRINKLER SYSTEM AND DESIGN NEW SYSTEM FOR ALL NEW VEGETATION. DESIGN AND MATERIALS TO BE APPROVED BY THE CITY PRIOR TO INSTALLATION.

SHRUBS	CODE	PROPOSED/EXISTING	QTY	BOTANICAL/COMMON NAME	SIZE
	SB	PROPOSED	18	SALVIA AZUREA 'BLUE SAGE'	5 GAL.
	BG	PROPOSED	8	BUXUS MICROPHYLLA 'GOLDEN TRUMPH'	5 GAL.
	--	EXISTING	3	----------	----

TREES	CODE	PROPOSED/EXISTING	QTY	BOTANICAL/COMMON NAME	CALIPER SIZE
	ZC	PROPOSED	6	ZELKOVA. CITY SPRITE (SERRATA)	6 IN.
	AM	PROPOSED	6	ACER GLABRUM 'MAPLE ROCKY MOUNTAIN'	5 IN.
	--	EXISTING	5	----------	--

1" CRUSHED ROCK FOR ISLANDS. COLORS AND STYLES TO BE SELCTED BY CLEARFIELD CITY. SUBMIT SAMPLE TO CITY FOR APPROVAL.

3" TO 6" COBBLE ROCK. COLORS AND STYLES TO BE SELECTED BY CLEARFIELD CITY. SUBMIT SAMPLE TO CITY FOR APPROVAL.

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MARCH, 21 2024

CLEARFIELD CITY PUBLIC WORKS COMPLEX PROJECT:

497 SOUTH MAIN STREET
CLEARFIELD, UTAH 84015

PROJECT NO. ----
DATE: MARCH, 21 2024

REVISIONS:
1. ADA SIDEWALK & STALL
2.
3.
4.
5.
6.
7.

SHEET TITLE:
LANDSCAPE PLAN

SHEET NUMBER:
C6.0

CIVIL

CIVIL DRAWINGS #1